

Load Bearing Framing Systems

**Super
Stud**
Building Products, Inc.



**The Structure
Behind
The Design**

Super Stud Building Products Technical Publications

Super Stud Building Products, Inc. is proud to introduce our series of technical publications for use as a guide to designing, specifying and installing our steel framing products.

We elected to furnish a series of technical publications in lieu of a single brochure. The load tables, specifications and installation drawings have been tailored to the specific application in question. This is offered to assist the end user who traditionally had to wade through unrelated text in an attempt to acquire pertinent information.

The available publications include:

- ◆ Load Bearing Framing Systems
- ◆ Curtainwall Framing Systems
- ◆ Curtainwall Deflection Solutions
- ◆ Drywall Framing Systems

Company Overview

Since 1973, **Super Stud Building Products, Inc.** has been a proud manufacturer of the industries most diverse offering of steel framing components and accessories for use in the construction of commercial, institutional and residential structures.

With its roots in the New York Tri-State area, **Super Stud** has successfully met the needs of some of the most demanding design professionals and contractors the building industry has to offer. Our commitment to quality products and prompt service has allowed us to expand to markets including the entire East Coast and beyond. **Our team of professionals in sales, customer service and production are committed to furnishing competitive pricing and timely deliveries for the full term of a project.**

Super Stud is instrumental in the development of cold formed steel framing systems. We offer a wide variety of standard sections to satisfy the requirements of any light structural or interior drywall application. We distinguish ourselves from our competition by furnishing a series of complementary accessories tailored to satisfy the needs of both designer and contractor. **Super Stud's** Product Development Staff subjects our system components to rigorous in-house and field testing procedures, when warranted and/or as required, to assure their load carrying capacities and compatibility for intended use.

Super Stud plays an active role in the steel framing industry. We are involved with the American Iron and Steel Institute (AISI), the Association of Walls and Ceilings International (AWCI) and the American Society for Testing and Materials (ASTM), Steel Framing Alliance (SFA).

Framing products manufactured by **Super Stud** are formed from steel coil meeting the minimum requirements of industry adopted ASTM Standards. This includes requirements for yield, ductility and protective coating. Our manufacturing facility has the latest equipment in shearing, roll forming, slitting, stamping and brake forming equipment enabling us to satisfy the most demanding production schedules without compromising quality.

Technical Services

Super Stud's Technical Support Group prides itself in offering a full complement of services that are tailored to the specific needs of the client. We are available to provide engineering and architectural support from the planning stages through final detailing for general and specific building conditions.

We can assist in providing certified structural calculations along with necessary drawings and specifications for any wall, floor, roof or truss assembly. We are also available to provide field follow-up services to assist the contractor in bringing about a successful completion of the project.

To obtain assistance, contact your local **Super Stud** Sales Representative who may be reached by calling our main office at 1-800-477-STUD (7883).

Table of Contents

Company Overview, Technical Services, Warranty & Limitations	3
Structural Framing Products	4
Custom Manufactured Products	6
Suggested Design Considerations	7
<i>This section serves as a general overview of design considerations in the evaluation of a steel framing system.</i>	
Section Properties	10
Suggested Design Guides	
<i>Tables include</i>	
- <i>Braced Studs Subjected to Lateral Load.</i>	<i>15-20</i>
- <i>Allowable Axial Load Capacities Unbraced Studs NOT Subjected to Lateral Load.</i>	<i>21</i>
- <i>Allowable Uniform Load Capacities Headers</i>	<i>22</i>
- <i>Allowable Uniform Load Capacities Joists</i>	<i>23-25</i>
- <i>Allowable Capacities Unstiffened Web(s) Subjected to Local Forces</i>	<i>26-27</i>
Suggested Framing Applications	28
<i>Describes various methods of framing a curtainwall system, from infill to by-pass walls. Includes suggested construction details and installation notes.</i>	
Guide Specification	35

Structural Framing Products

General Information

◆ Mechanical Properties, Base Steel

Unless noted otherwise herein, structural framing components manufactured by **Super Stud** are formed from steel meeting the requirements of ASTM A1003 and the following specifications:

	ASTM A1003 (F _y (min) = 50 KSI)	ASTM A1003 (F _y (min) = 33 KSI)
Studs	97, 68 & 54* mils	43 & 33 mils
Track & Accessories	97* & 68* mils	54*, 43 & 33 mils

F_y = Minimum Yield Point

* F_y=33KSI, 50 KSI must be specified at time of order.

ASTM A1003, entitled Standard Specification for Steel Sheet, Zinc Coated (Galvanized) by the Hot-Dip Process, Structural (Physical) Quality, covers sheet steel of structural (physical) quality with zinc coating.

◆ Galvanized Coating

Super Stud structural framing components are zinc coated (galvanized) in accordance with ASTM A653 and ASTM A1003. Super Stud can also fulfill requests for G-90 coatings which provides 50 percent more zinc protection.

◆ Design Steel Thickness

The structural properties and load tables were prepared using the following design steel thicknesses:

20 gage: 33 mils, 0.0346 inch	18 gage: 43 mils, 0.0451 inch
16 gage: 54 mils, 0.0566 inch	14 gage: 68 mils, 0.0713 inch
12 gage: 97 mils, 0.1017 inch	

In conformance with the AISI Specification and AISI-S200-07, the actual delivered base steel thickness, individual measurement, must not be less than 95 percent of the inch values listed above.

◆ Identification

Super Stud identifications and markings conform to AISI-S200-07 and ASTM A1003 identification.

PRODUCT MATRIX

Section/Flange Width	2-1/2"	3-1/2"	3-5/8"	4"	5-1/2"	6"	7-1/4"	8"	9-1/4"	10"	11-1/2"	12"	14"	16"
SSCW / 1-1/4" FLG.			33 mils	33 mils		33 mils								
SSC / 1-3/8" FLG.	33-68 mils	33-68 mils	33-68 mils	33-68 mils	33-68 mils	33-68 mils								
SSJ / 1-5/8" FLG.	33-68 mils	33-68 mils	33-68 mils	33-68 mils	33-68 mils	33-97 mils	43-97 mils	43-97 mils	43-97 mils	54-97 mils	54-97 mils	54-97 mils		
SJW / 2" FLG.			43-97 mils	43-97 mils	43-97 mils	43-97 mils	43-97 mils	43-97 mils	43-97 mils	54-97 mils	54-97 mils	54-97 mils	54-97 mils	
SSW / 2-1/2" FLG.				43-97 mils		43-97 mils		43-97 mils		54-97 mils	54-97 mils	54-97 mils	68-97 mils	68-97 mils
SSX / 3" FLG.			54-97 mils	54-97 mils	54-97 mils	54-97 mils	54-97 mils	54-97 mils	54-97 mils	54-97 mils	54-97 mils	54-97 mils	68-97 mils	68-97 mils
SSXW / 3-1/2" FLG.						54-97 mils	54-97 mils	54-97 mils	54-97 mils	54-97 mils	54-97 mils	54-97 mils	68-97 mils	68-97 mils
TR / 1-1/4" FLG.	33-68 mils	33-68 mils	33-68 mils	33-68 mils	33-68 mils	33-68 mils	33-68 mils	33-68 mils	43-68 mils	43-97 mils	54-97 mils	54-97 mils	68-97 mils	68-97 mils
TW / 2" FLG.			33-68 mils	33-68 mils		33-97 mils		43-97 mils		54-97 mils				
DT / 2-1/2" FLG.			54-97 mils	54-97 mils		54-97 mils		54-97 mils		54-97 mils				

DISCLAIMER:

All data, specifications and details contained in this publication are intended as a general guide for using Super Stud products. These products should not be used in design or construction without an independent evaluation by a qualified engineer or architect to verify the suitability of a particular product for use in a specific application. Super Stud assumes no liability for failure resulting from the use or misapplication of computations, detail drawings and specifications contained herein. This publication contains the latest information available at the time of printing. Super Stud reserves the right to make modifications and/or change materials of any of their products without prior notice or obligation. For the latest information regarding a particular manufacturer's products contact that manufacturer. Please contact Super Stud to verify product availability. The Design Professional is suggested to contact Super Stud to assure that there are no significant changes of information after the printing of this catalog.

The intent of this publication is to furnish the design professional with information allowing for the evaluation of a cold formed steel framed assembly. Due to the variable nature of a structure, this publication serves only as a suggested guide to the use of our steel framing sections and accessories. As such, the user is cautioned to thoroughly evaluate any assembly for specific performance and installation requirements.

The drawings, specifications and technical tables are furnished to assist in the evaluation of these products by a qualified structural engineer or architect in their application or installation.

Super Stud makes no recommendations or representations as to their use or application other than that the products will comply with the specifications within industry tolerances.

Super Stud assumes no liability for damages resulting from improper application or installation.

WARRANTIES AND LIMITATIONS

All products presented herein are warranted to be free from manufacturing defects in material and workmanship. Damages for the breach of this warranty and buyers sole remedy shall be the return of the goods and the repayment of the price thereof or the replacement of such non-conforming goods at the sole option of the seller. Seller specifically excludes any liability for any consequential damages as a result of the non-conformance of the goods.

The aforesaid warranty is exclusive to the direct purchaser from Super Stud and is not assignable or transferable to any third party. This warranty is in lieu of and EXCLUDES ALL OTHER WARRANTIES NOT EXPRESSLY SET FORTH HEREIN, whether express, implied or by operation of the law. Specifically EXCLUDED ARE ANY IMPLIED WARRANTY OF MERCHANTABILITY OR FITNESS FOR A PARTICULAR PURPOSE.

Super Stud denies any liability for damages resulting from the use of the details or specifications or improper application or installation of these products.

GOVERNING LAW

The parties hereby consent to the jurisdiction of the Superior Court of New Jersey, Middlesex County, as having sole jurisdiction over disputes arising out of this sales transaction, including, but not limited to, the non-payment for any sale made or claim of an alleged Breach of Warranty. It shall not be a defense to any action brought by the seller that the aforesaid Courts do not have jurisdiction over the purchase.

New Jersey Law shall apply in all such actions.

The Statute of Limitations for any such action based upon an alleged breach shall be one (1) year from the date of the alleged breach and no action may be maintained if brought after the said one (1) year. The alleged breach shall be deemed to have occurred at the time of the delivery of merchandise.

Structural Framing Products

Studs (SSCW, SSC, SSJ, SJW, SSW, SSX & SSXW)

Studs serve as structural elements in the construction of exterior curtainwalls, soffits, load bearing walls and headers. They are also used in floor joist, roof rafter and truss frame assemblies.

"C" studs are defined by three basic components...the web, flange and return lip. The flange serves to stiffen the web while the return lip stabilizes the flange.

Return Lip Length

Section	Flange Width	20 Ga	18 Ga	16 Ga	14 Ga	12 Ga	10 Ga
SSCW	1-1/4"	0.250"	NA	NA	NA	NA	NA
SSC	1-3/8"	0.375"	0.375"	0.375"	0.375"	NA	NA
SSJ	1-5/8"	0.5"	0.5"	0.5"	0.5"	0.5"	0.5"
SJW	2"	NA	0.625"	0.625"	0.625"	0.625"	0.625"
SSW	2-1/2"	NA	0.625"	0.625"	0.625"	0.625"	0.625"
SSX	3"	NA	1.000"	1.000"	1.000"	1.000"	1.000"
SSXW	3-1/2"	NA	1.000"	1.000"	1.000"	1.000"	1.000"

Web Knockouts

Studs are typically furnished with punched webs to facilitate the installation of conduit, piping and bridging. The knockout sizes and locations are defined below. Members may be furnished with unpunched webs upon request.

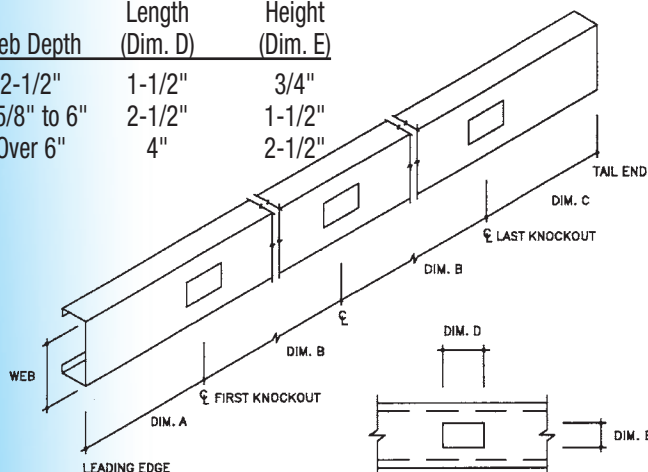
Knockout Spacing

Section	First Knockout (Dim. A)	Typical Spacing (Dim. B)	Last Knockout (Dim. C) ^{Note 1}
All sections except those listed below	24"	24"	12" min.
8" SSJ - 12" SSJ			
8" SJW - 14" SJW	48"	48"	24" min.
8" SSW - 16" SSW			

Note 1: 24" Min. applies to millcut sections only. Does not apply to components saw cut from stock.

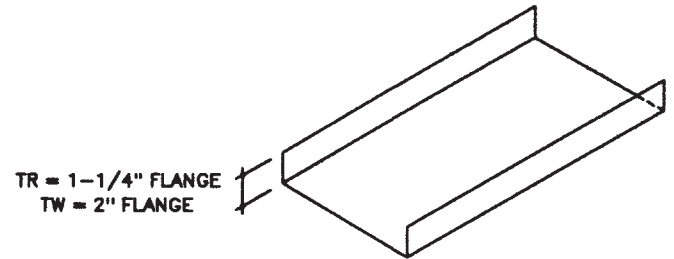
Knockout Size

Web Depth	Knockout Length (Dim. D)	Knockout Height (Dim. E)
2-1/2"	1-1/2"	3/4"
3-5/8" to 6"	2-1/2"	1-1/2"
Over 6"	4"	2-1/2"



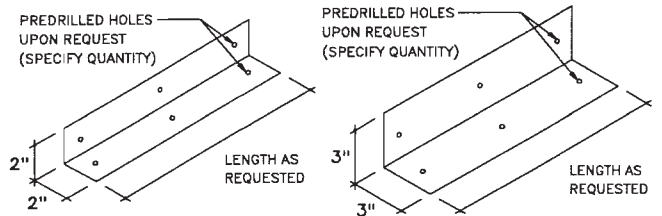
Track (TR & TW)

Track is used as an enclosure for studs in the construction of wall and joist assemblies. Track also serves as a structural component in the design of sill, head and jamb assemblies of framed openings.



Angle Clips (AC)

Thickness Offering: 33, 43, 54, 68 and 97 mils



Angle Clips serve a variety of functions in the construction of a steel framing system. They may be used to make attachments between framing members or, in the case of curtainwall construction, to transfer gravity and lateral loads from a stud to the primary frame.

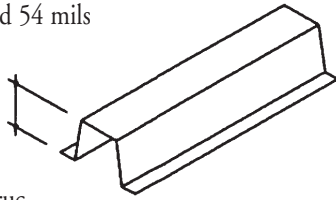
Angle Clips are available in standard 2" or 3" leg dimensions. Non-standard angles with varying leg dimensions are also available. Upon request, Angle Clips can be furnished pre-punched to facilitate screw attachments.



◆ Structural Hat Channel (SHC)

Thickness Offering: 33, 43 and 54 mils

Structural Hat Channel is used to 1-1/2" provide support for sheathing, siding and decking products in the construction of soffits, ceilings, roofs and walls. The channel is compatible for use over any of a number of different support members.



◆ Utility Angle (UA)

Utility Angles, furnished in standard 10 foot lengths, are typically installed at corner framing conditions to provide continuous support for sheathing products.

Utility Angles are available in standard 2" or 3" leg dimensions. Non-standard angles with varying leg dimensions are also available.

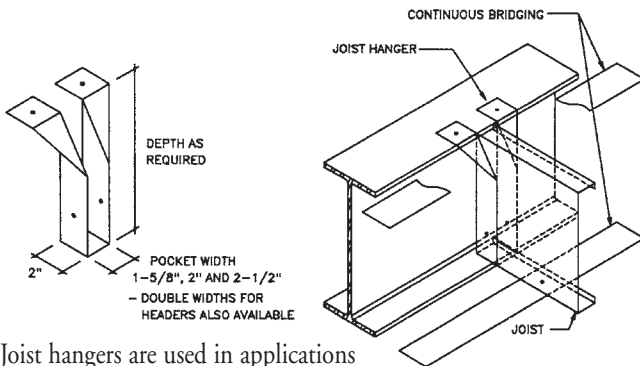
◆ Tension Strap (TS)

Thickness Offering: 33, 43, 54, 68 and 97 mils

Tension Strap has a variety of uses some of which include wall and joist bridging, shear walls and backer plates to facilitate the installation of handrails and fixtures.

Joist Hangers (JH)

Thickness Offering: 97 mils



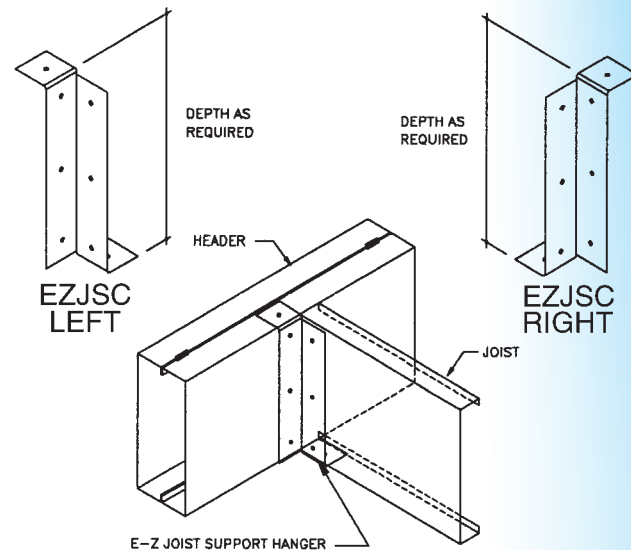
Joist hangers are used in applications where vertical alignment of the joist and the support steel is desired. The hanger is available in single or double widths to accommodate single joists or header assemblies.

Notes:

1. Joist web knockouts shall not occur within one foot of the Joist Hanger. The joist reaction shall not exceed the allowable capacity of the unstiffened web. (Reference Pages 29 and 30 for capacities.) A minimum joist end bearing width of two inches shall be provided.
2. The Joist Hanger shall be attached to the steel support. Acceptable fastening methods include:
 - a. Two (2) 0.145" or 0.140" diameter knurl shanked powder actuated fasteners. The shank shall be of a length to assure tip penetration through the beam flange.

- b. Two (2) No. 12 diameter X 24 Threads Per Inch self drilling screws. The cutting tip of the screw shall accommodate the combined thickness of the hanger and the beam flange.
 - c. Four (4) 1" long welds (total), two (2) each side.
3. Reference Detail M2, Page 37, for additional information.

◆ EZ Joist Support Clip (EZJSC) (Patent Pending)



Thickness Offering: 54 and 68 mils

The EZ Joist Support Clip is distinguished by its return leg which positions the joist in place during installation. Simply rest the joist on the return leg and make the necessary screw attachments. It eliminates time consuming joist leveling and clamping steps required when a member is attached using traditional clip angles.

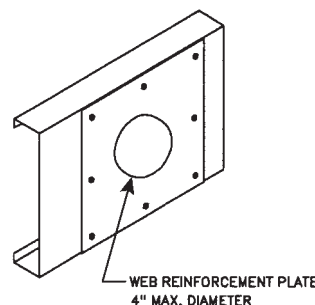
Left and right side clips are available to correspond to the desired joist web position and in depths to accommodate all "C" sections up to 12 inches in depth.

Notes:

1. A minimum of six (6) fasteners, three (3) each leg, shall be used to attach the clip to the joist and support steel. Fastener types and sizes shall be specified by the engineer/architect of record.
2. The leg of the clip projecting beneath the joist shall not be used to support joist reactions.
3. Contact Super Stud's Technical Services Department for allowable load capacities.

◆ Web Reinforcement Plate (WRP)

Thickness Offering: 43, 54, 68 and 97 mils



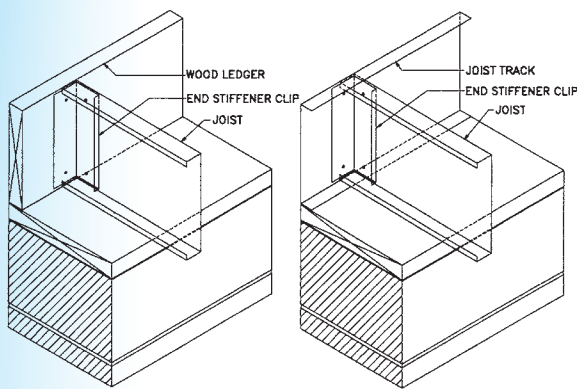
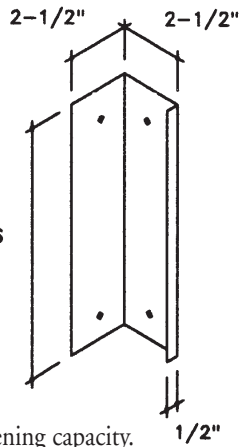
Steel plates available in a range of sizes which are used to reinforce web cut-outs exceeding the dimensions of our standard web punchout offering.

◆ End Stiffener Clip (ESC)

Thickness Offering: 54 and 68 mils

The End Stiffener provides web reinforcement while it functions as an attachment clip in the construction of a joist assembly. The clip is distinguished by its extended flange which provides ample surface area for attachment to a continuous steel track or wood ledger and its return lip which increases its stiffening capacity.

LENGTH AS REQUIRED

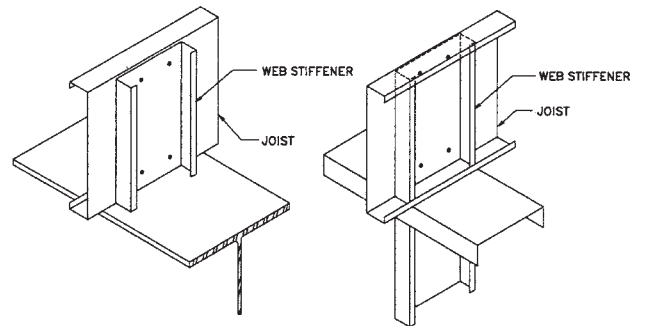
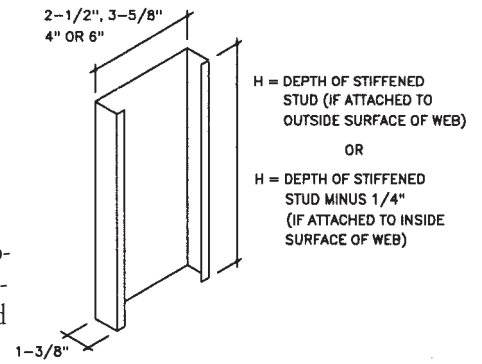


◆ Web Stiffener (WS)

Thickness Offering: 43, 54 and 68 mils

Web Stiffeners provide web reinforcement for studs and joists at both end and intermediate

support or concentrated load locations. The stiffener, placed either inside or outside the web, is manufactured on a custom order basis. Stiffeners located "inside" the web should be ordered to lengths equalling the depth of the member less 1/4" while those installed "outside" the web should match the depth of the member. The width of the stiffener should match the width of bearing above and/or below the stiffener.



Custom Manufactured Products

The use of conventional straight track to achieve a radius is a time consuming venture which requires notching, folding and re-attaching the track to the required formation.

Super Stud has the capability to manufacture Radius Track formed of two rolled and curved angles and splice plates. Eliminate the costs of segmenting track by using one continuous section curved to meet your needs.

◆ Brake Formed Accessories

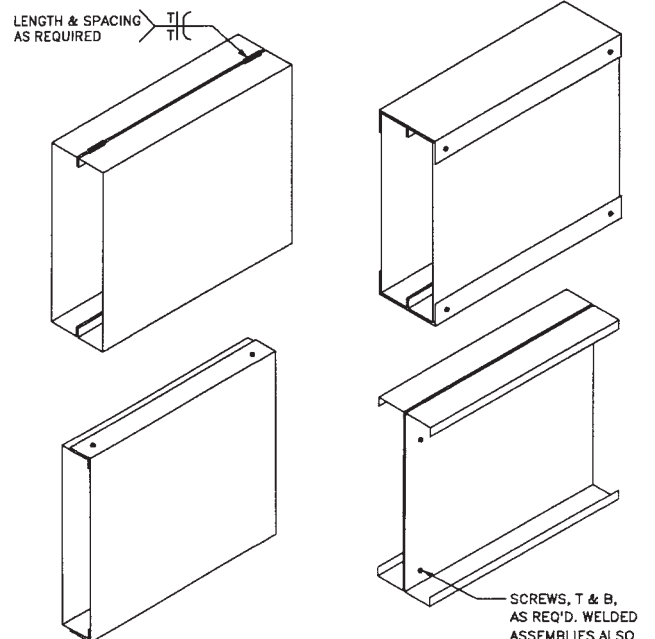
At **Super Stud**, if its made from steel, we'll find a way to shear, brake, roll or punch it to satisfy any unique framing requirement. From custom brake formed closures to radius track, our sales, customer service, production and technical representatives are available to discuss any of your framing needs.

◆ Sheet Steel

Super Stud can furnish de-coiled Sheet Steel in widths up to 48 inches. They may be used for a variety of applications including security shields between studs and facing materials.

◆ Boxed and "I" Shaped Assemblies

To reduce field labor, Super Stud provides factory assembled boxed and "I" shaped sections for use as posts, jambs, headers and beams. They are manufactured on a custom order basis to meet project requirements. Welded or screw attached assemblies are available.



Suggested Design Considerations

The design of the steel framing system shall be based on the requirements of the latest edition of the American Iron and Steel Institute (AISI) North American Specification for the Design of Cold Formed Steel Structural Members. In addition to the specification, the manual includes applicable commentary, load tables, design examples and references.

To obtain a copy of the Cold Formed Steel Design Manual, contact

The American Iron and Steel Institute
1140 Connecticut Ave., NW Ste. 705
Washington, D.C. 20036
202-452-7100
www.steel.org

Stress Analysis

◆ Forward

The North American Specification contains provisions for both Allowable Strength Design (ASD) and Load and Resistance Factor Design (LRFD) methods. For the ASD approach, the sections design capacities (allowable capacities) are determined by dividing the sections nominal capacities by a predetermined safety factor. For the LRFD approach, the design capacities (resistance capacities) are determined by multiplying the sections nominal capacities by a predetermined resistance factor (ϕ (Φ) factor). The North American Spec no longer permits the 1/3 stress increase (or a 0.75 load combination reduction factor) for load combinations containing wind and/or seismic loads.

◆ Bending Moment, $M_{actual} (K-in) \leq M_{allow} (K-in)$

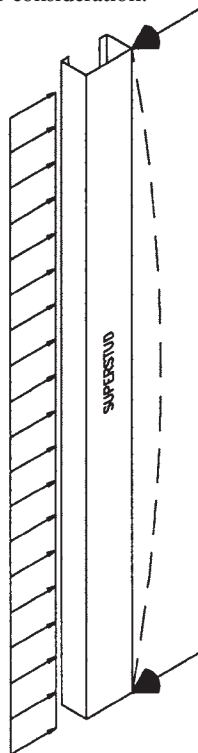
The applied bending moment shall not exceed the allowable bending moment capacity of the section under consideration.

The allowable bending moment capacities are based on ASD method.

Studs shall be braced against rotation by diaphragm rated sheathing materials applied full height to each side of the wall. The installation of mechanical bridging spaced at intervals not to exceed 5'-0" on center provides adequate rotation restraint for walls under construction before the installation of sheathing materials.

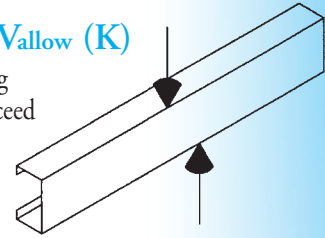
In applications where the wall is not sheathed full height or sheathed on one side only, continuous bridging spaced not to exceed 5'-0" on center shall provide rotational support. Reductions in allowable bending capacity must be investigated separately.

If sheathing products are considered as a method of bracing, they must maintain their integrity during the course of construction and the service life of the wall.

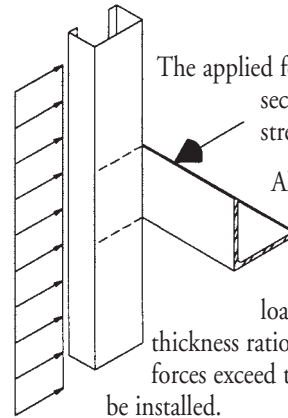


◆ Shear, $V_{actual} (K) \leq V_{allow} (K)$

The applied shear force acting through the web shall not exceed the allowable shear of the section. Shear capacities are based on ASD method.



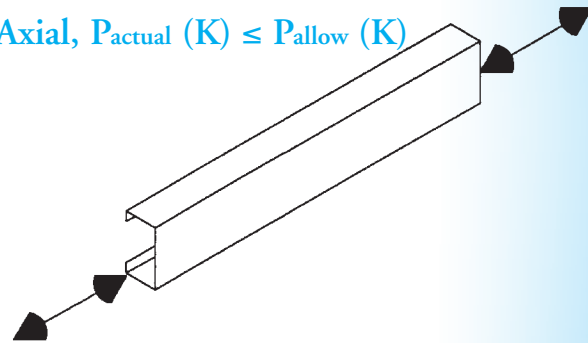
◆ Web Crippling, $P_{actual} (K) \leq P_{allow} (K)$.



The applied forces acting through the web of a section shall not exceed the allowable strength of the unreinforced web.

Allowable capacities are based on ASD method. Items to be considered include bearing width, one versus two flange loading, steel grade and depth to thickness ratios of the web. Where the applied forces exceed the allowable, a web stiffener shall be installed.

◆ Axial, $P_{actual} (K) \leq P_{allow} (K)$



Axial force is based on ASD method.

Axial loads developed through a steel framed curtainwall system are generally limited to the gravity (dead) weights of the stud framing and the collateral materials attached or suspended from it.

Attention should be given to wall installations in which provisions for primary frame deflections are not made. An infill wall built tight to the underside of a structure, for example, is subjected to axial stresses when the primary frame deflects due to live load, support settlement, etc. While the framing member may be analyzed to withstand the applied axial load, consideration should be given to the difficulty associated with seating stud ends in pre-attached tracks.

Tension forces may also occur. Examples include kickers and struts used to resist lateral negative loads acting away from the wall or element.

◆ Combined Stresses

The design of the wall studs shall be based on the following design wind loading considerations in accordance with the AISI S211-07 (North American Standard for Cold-Formed Steel Framing – Wall Stud Design):

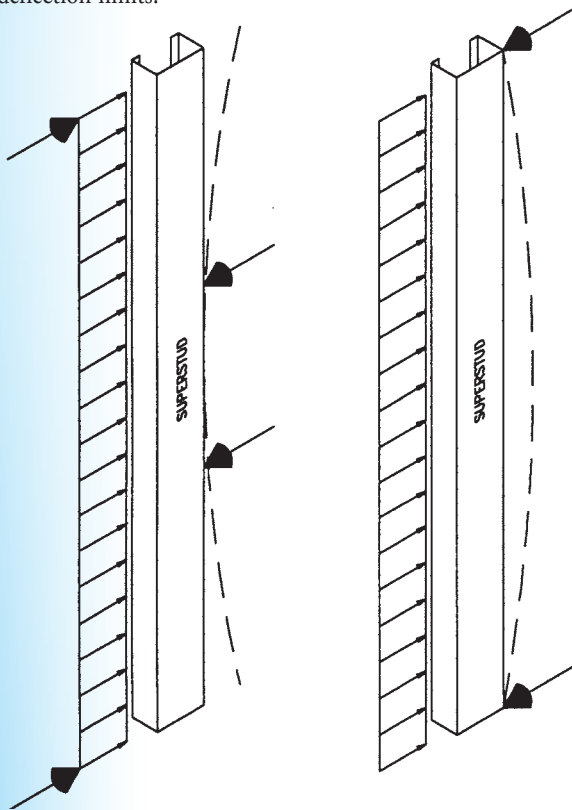
- Combined bending and axial strength based on Main Wind Force Resisting System (MWFRS) wind loads.
- Bending strength based on Components and Cladding (C&C) wind loads.
- Deflection limits based on 70% of Components and Cladding (C&C) wind loads with no axial loads.

Deflection

◆ Wall Deflection

Limiting deflection of a load bearing wall is subjected to lateral load is critical to the performance of the finish system, sealants and components anchored to the framing system.

There are a number of exterior finish systems which may be used in conjunction with a steel stud wall, and as such, the allowable deflection of the framing may vary. Consult technical literature prepared by the finish system manufacturer or applicable industry standards for determination of allowable deflection limits.



A deflection analysis of the head, sill and jamb components at wall openings is also required. Door, window and louver manufacturers may be contacted for allowable movements of the support framing.

Traditionally, deflection analysis is based on the properties of the framing acting independently without contribution due to the attachment of collateral materials.

In most cases, the selection of a loading bearing wall stud is controlled by deflection analysis, particularly in applications involving rigid facing materials such as brick veneer.

The moment of inertia, I_x (in⁴), found in the Section Property Tables, is the critical variable used in determining the deflection of a stud subjected to lateral load. Realizing that the moment of inertia is taken about an axis that passes through the center of the area, the farther the component parts of an area are from the axis, the greater the value of moment of inertia. In general terms, increasing the depth of a section will have a greater effect on a member's ability to limit deflection than will an increase in mil thickness.

Acknowledging that the area of an element is directly related to its costs, consider the following comparison:

Stud Type	Moment of Inertia I (in ⁴)	Area (in ²)
358 SSJ 14 Ga (68 mils)	1.069	0.524
6 SJW 18 Ga (43 mils)	2.686	0.492

The 6" stud provides approximately 2-1/2 times the stiffness of the 3-5/8" member using an equivalent area of steel.

We note that stud selection may be governed by any of a number of design factors beyond limiting deflection such as stress, connection strength, depth limitations, etc. and as such each condition should be closely reviewed for compliance with design requirements.

◆ Deflection of the Primary Frame

A curtainwall system should be constructed in a manner that allows for deflection of the primary building frame. **Super Stud** furnishes a series of accessories to accommodate frame deflections while they provide support for lateral wind loads.

Infill walls should be constructed incorporating a Deflection Track which accommodates primary frame movements while it braces the member against lateral loads. Where walls by-pass the primary frame, **Super Stud's** Deflection Clip or Strut should be specified.



Connections

◆ Forward

The attachment of steel framing may be accommodated by any of a number of different fastener types or methods. Connections shall be designed in accordance with the provisions of the AISI Specification supplemented with the selected manufacturer's published load values and/or industry standards.

Commonly used methods of fastening steel framing components include:

◆ Screws

Self-drilling screws provide an efficient method of transferring shear forces between framing members or combined shear and tensile forces between framing members and structural steel elements.

Screws shall be of proper length to assure complete penetration of the cutting tip before the threads engage the steel of the joined materials. The flute on the cutting tip determines the metal thickness which can be drilled as it provides a channel for chip removal during drilling. Where screw attachments are made to framing components of different thicknesses, attachment through the thinner component to the heavier is required. Screw penetration through joined materials shall not be less than three exposed threads.

Design considerations include tension and shear capacity of the screw in addition to the bearing and pullover capacity of the base steel component.

◆ Welds

The design of welded connections shall be performed in accordance with the American Welding Society AWS D1.3, Structural Welding Code-Sheet Steel. Provisions of this code were adopted in the AISI Specification for the Design of Cold Formed Steel Structural Members.

To obtain a copy of the code contact:

Order Department
American Welding Society
550 N.W. LeJuene Road
Miami, Florida 33126
800-443-9353
www.aws.org

Design and performance of a welded connection is based on the evaluation of the following:

- intensity and direction of loads
- weld type (fillet, groove, spot or seam)
- material grade of the base steel or filler
- welding methods and recommended practices
- treatment of welds and the surrounding steel areas to restore its weatherability

Field welds involving 20 gage materials should be avoided.

◆ Power Actuated Fasteners to Concrete

Power actuated fasteners used for the attachment of framing to concrete shall be of adequate length to assure minimum embedment requirements. Consideration shall be given to minimum edge distance and spacing between fasteners along with the base thickness and compressive strength of the concrete.

Failure modes include pullout and shear of the fastener and pullover or bearing of the fastened material. These fasteners shall be avoided in conditions subjected to excessive tension or vibration loads.

◆ Expansion Bolts or Adhesive Anchors to Concrete

Similar to the power actuated fastener, the performance of an expansion bolt is affected by its length of embedment, edge distance and spacing between fasteners along with base material thickness and grade of concrete.

Adhesive bolts, while more expensive than the expansion bolt, offer high load capacity and good performance under dynamic load without exerting expansion pressure on the base material.

◆ Power Actuated Fasteners to Structural Steel

A power actuated fastener driven into steel is influenced by the thickness and tensile strength of the base steel along with fastener spacing and edge distance.

The fastener shall be adequate length to ensure tip penetration through the base steel component. Knurled shank fasteners increase its tensile capacity. Tension capacities are generally limited to the pullover strength of the fastened materials. To increase pullover capacity, steel washers may be specified.

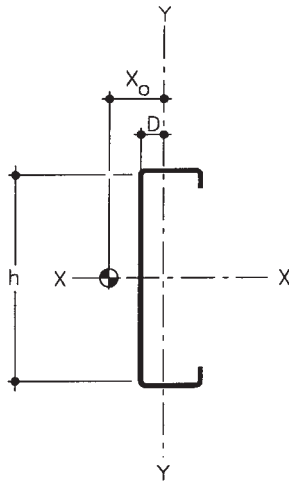
Fire Rated Assemblies

Steel framed assemblies finished with gypsum sheathing and wallboard provide up to four hour fire resistance ratings. Reference the Fire Resistance Design Manual published by the Gypsum Association or Underwriters Laboratories Fire Resistance Directory for specific information on rated wall, floor and roof assemblies.

Fire Resistance Directory
Underwriters Laboratories, Inc.
Publications Stock
333 Pfingsten Road
Northbrook, Illinois 60062
877-854-3577
www.ul.com

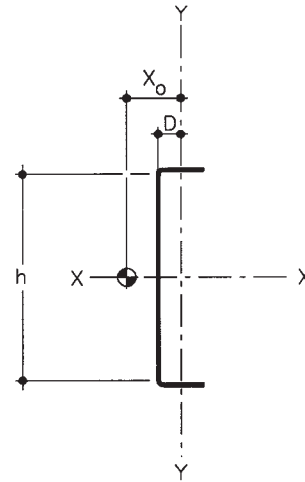
Fire Resistance Design Manual
Gypsum Association
6525 Belcrest Road, Suite 480
Hyattsville, MD 20782
301-277-8686
www.gypsum.com

SECTION PROPERTIES



STUD
FLANGE WIDTH

COMPONENT	WIDTH
SSCW	1-1/4"
SSC	1-3/8"
SSJ	1-5/8"
SJW	2"
SSW	2-1/2"
SSX	3"
SSXW	3-1/2"



TRACK
FLANGE WIDTH = 1-1/4" (TR)

Terms and Definitions

- Weight Weight per lineal foot, Plf
- Ma Allowable bending moment of braced section, K-in
- Va Allowable shear force through an unpunched web, Kip
- Area Cross-sectional area of gross section, in²
- I_x, I_y Moment of inertia of gross section about applicable axis, in⁴
- S_{xe}, S_{ye} Section modulus of the effective section stressed at yield about the applicable axis, in³
- R_x, R_y Radius of gyration of gross section about applicable axis, in
- D Distance from the Y axis to outside of web, In
- J_x 1000 St. Venant torsional constant, in⁴, multiplied by 1000
- C_w Torsional warping constant, in⁶
- R_o Polar radius of gyration about the shear center, in
- X_o Distance from shear center to centroid along the principal axis, in
- Beta $1-(X_o/R_o)^2$
- h/t Flat web to thickness ratio

Notes:

- Section properties were prepared in accordance with the North American Specification for the Design of Cold Formed Steel Structural Members, 2007 addition.

- Allowable bending moment, Ma, was calculated in accordance with AISI Section C3.1, Procedure 1, based on the initiation of yield in the effective section.
- Bearing stiffeners are required for all components where the h/t ratio exceeds 200.
- S_{xe} and S_{ye} are based on the effective section stressed at yield. Reference AISI Section B2.
- Ma & Va are based on steel meeting the minimum requirements of the following specifications:

	ASTM A1003 (F _y (min)= 50 KSI)	ASTM A1003 (F _y (min)= 33 KSI)
Studs	97, 68 & 54* mils	43 & 33 mils
Track & Accessories	97* & 68* mils	54*, 43 & 33 mils

F_y = Minimum Yield Point

* F_y=33KSI, 50 KSI must be specified at time of order.

Upon request, Super Stud will fulfill requests for any of our components manufactured from steel meeting the minimum requirements of ASTM A1003, Grade B, F_y(min)=37 KSI and Grade C, F_y(min)= 40 KSI.

- The structural properties and load tables were prepared using the following base steel thicknesses:

33 mils: 0.0346 inch	43 mils: 0.0451 inch	54 mils: 0.0566 inch
68 mils: 0.0713 inch	97 mils: 0.1017 inch	

In conformance with the AISI Specification, the actual delivered base steel thickness, individual measurement, must not be less than 95 percent of the values listed above.

SECTION PROPERTIES

$2\frac{1}{2}" - 4"$

SSCW, SSC, SSJ, SJW & SSW STUDS

Load Bearing Framing Systems

Section	Weight P/f	Ma K-in	Va Kip	Area in ²	I _x in ⁴	S _{xg} in ³	R _x in	I _y in ⁴	S _{yg} in ³	R _y in	D in	JX1000 in ⁴	C _w in ⁶	R _o in	X _o in	Beta	h/t
2-1/2SSC20	0.689	3.263	1.037	0.202	0.208	0.165	1.013	0.056	0.060	0.526	0.501	0.081	0.086	1.666	-1.213	0.470	66.642
2-1/2SSJ20	0.762	3.557	1.037	0.224	0.237	0.188	1.028	0.088	0.082	0.625	0.628	0.089	0.145	1.920	-1.497	0.392	66.642
2-1/2SSC18	0.879	4.134	1.354	0.258	0.263	0.209	1.009	0.069	0.074	0.516	0.492	0.175	0.101	1.637	-1.181	0.480	50.432
2-1/2SSJ18	0.994	4.780	1.354	0.292	0.304	0.242	1.020	0.114	0.109	0.624	0.637	0.198	0.194	1.927	-1.511	0.385	50.432
2-1/2SSC16	1.099	7.694	2.510	0.323	0.323	0.257	1.001	0.085	0.093	0.514	0.500	0.345	0.130	1.643	-1.197	0.469	39.170
2-1/2SSJ16	1.243	8.897	2.510	0.365	0.373	0.297	1.011	0.141	0.137	0.622	0.645	0.390	0.250	1.935	-1.528	0.376	39.170
2-1/2SSC14	1.312	9.097	3.057	0.386	0.382	0.304	0.996	0.092	0.096	0.489	0.469	0.653	0.125	1.574	-1.117	0.497	30.063
2-1/2SSJ14	1.509	10.724	3.057	0.443	0.450	0.358	1.008	0.162	0.154	0.605	0.625	0.751	0.262	1.885	-1.474	0.389	30.063
3-1/2SSC20	0.806	4.992	1.046	0.237	0.452	0.253	1.381	0.063	0.060	0.515	0.431	0.095	0.168	1.827	-1.080	0.651	95.543
3-1/2SSJ20	0.880	5.424	1.046	0.259	0.511	0.286	1.406	0.099	0.082	0.618	0.547	0.103	0.273	2.043	-1.348	0.565	95.543
3-1/2SSC18	1.032	6.336	1.762	0.303	0.574	0.321	1.375	0.077	0.075	0.505	0.422	0.206	0.200	1.802	-1.050	0.661	72.605
3-1/2SSJ18	1.147	7.305	1.762	0.337	0.660	0.370	1.399	0.129	0.110	0.618	0.555	0.229	0.363	2.046	-1.360	0.558	72.605
3-1/2SSC16	1.291	11.856	3.417	0.379	0.709	0.396	1.367	0.096	0.094	0.503	0.429	0.405	0.254	1.803	-1.063	0.653	56.837
3-1/2SSJ16	1.436	13.670	3.417	0.422	0.815	0.457	1.390	0.160	0.139	0.616	0.563	0.451	0.463	2.050	-1.375	0.550	56.837
3-1/2SSC14	1.554	14.037	4.483	0.457	0.841	0.469	1.356	0.104	0.099	0.476	0.401	0.774	0.258	1.744	-0.988	0.679	44.088
3-1/2SSJ14	1.752	16.502	4.483	0.515	0.985	0.551	1.383	0.184	0.157	0.597	0.543	0.872	0.503	2.004	-1.321	0.565	44.088
3-5/8SSCW20	0.777	4.843	1.008	0.228	0.454	0.245	1.410	0.048	0.048	0.456	0.360	0.091	0.130	1.743	-0.916	0.723	99.156
3-5/8SSC20	0.821	5.240	1.008	0.241	0.490	0.265	1.426	0.064	0.060	0.514	0.423	0.096	0.180	1.853	-1.066	0.669	99.156
3-5/8SSJ20	0.895	5.691	1.008	0.263	0.554	0.300	1.452	0.100	0.082	0.617	0.538	0.105	0.293	2.064	-1.331	0.584	99.156
3-5/8SSC18	1.052	6.652	1.762	0.309	0.623	0.337	1.420	0.078	0.075	0.503	0.415	0.210	0.215	1.828	-1.035	0.679	75.377
3-5/8SSJ18	1.167	7.662	1.762	0.343	0.716	0.388	1.445	0.130	0.111	0.616	0.546	0.232	0.390	2.067	-1.344	0.577	75.377
3-5/8SJW18	1.311	8.715	1.762	0.385	0.837	0.455	1.474	0.227	0.163	0.768	0.728	0.261	0.726	2.418	-1.757	0.472	75.377
3-5/8SSC16	1.315	12.454	3.417	0.387	0.770	0.416	1.411	0.097	0.094	0.502	0.422	0.413	0.274	1.828	-1.049	0.671	59.046
3-5/8SSJ16	1.460	14.346	3.417	0.429	0.884	0.479	1.436	0.162	0.139	0.615	0.554	0.458	0.496	2.070	-1.358	0.570	59.046
3-5/8SJW16	1.676	16.415	3.417	0.493	1.048	0.569	1.459	0.299	0.221	0.778	0.763	0.526	1.063	2.473	-1.840	0.447	59.046
3-5/8SSC14	1.585	14.752	4.661	0.466	0.913	0.493	1.400	0.105	0.099	0.474	0.394	0.789	0.278	1.770	-0.974	0.697	45.842
3-5/8SSJ14	1.782	17.324	4.661	0.524	1.069	0.579	1.429	0.186	0.158	0.596	0.535	0.887	0.540	2.024	-1.305	0.585	45.842
3-5/8SJW14	2.055	20.770	4.661	0.604	1.277	0.694	1.454	0.351	0.259	0.762	0.744	1.023	1.175	2.427	-1.787	0.458	45.842
3-5/8SJW12	2.723	27.148	6.339	0.800	1.672	0.907	1.446	0.407	0.284	0.713	0.683	2.759	1.160	2.291	-1.628	0.495	30.644
4SSCW20	0.821	5.558	0.909	0.241	0.572	0.281	1.540	0.049	0.048	0.451	0.342	0.096	0.161	1.830	-0.880	0.769	109.994
4SSC20	0.865	6.003	0.909	0.254	0.617	0.304	1.558	0.066	0.060	0.508	0.403	0.101	0.222	1.933	-1.025	0.719	109.994
4SSJ20	0.939	6.510	0.909	0.276	0.696	0.343	1.588	0.103	0.083	0.612	0.513	0.110	0.359	2.132	-1.285	0.637	109.994
4SSC18	1.109	7.627	1.762	0.326	0.785	0.386	1.552	0.081	0.075	0.497	0.394	0.221	0.266	1.910	-0.996	0.728	83.692
4SSJ18	1.224	8.759	1.762	0.360	0.899	0.443	1.581	0.135	0.111	0.612	0.522	0.244	0.475	2.134	-1.297	0.631	83.692
4SJW18	1.368	9.940	1.762	0.402	1.049	0.518	1.615	0.235	0.164	0.765	0.698	0.273	0.877	2.468	-1.702	0.524	83.692
4SSW18	1.541	10.852	1.762	0.453	1.235	0.611	1.652	0.413	0.237	0.955	0.929	0.307	1.598	2.924	-2.215	0.426	83.692
4SSC16	1.387	14.299	3.417	0.408	0.971	0.478	1.543	0.100	0.095	0.496	0.402	0.435	0.337	1.909	-1.008	0.721	65.671
4SSJ16	1.532	16.423	3.417	0.450	1.113	0.549	1.572	0.168	0.140	0.610	0.529	0.481	0.603	2.136	-1.310	0.624	65.671
4SJW16	1.749	18.764	3.417	0.514	1.317	0.650	1.601	0.310	0.223	0.776	0.732	0.549	1.269	2.519	-1.783	0.499	65.671
4SSW16	1.977	20.506	3.417	0.581	1.552	0.766	1.634	0.548	0.326	0.971	0.975	0.621	2.396	3.004	-2.326	0.400	65.671
4SSC14	1.676	16.960	5.196	0.492	1.153	0.566	1.530	0.108	0.101	0.468	0.375	0.835	0.347	1.853	-0.935	0.746	51.101
4SSJ14	1.873	19.851	5.196	0.550	1.346	0.663	1.564	0.192	0.161	0.591	0.511	0.933	0.662	2.092	-1.258	0.639	51.101
4SJW14	2.146	23.733	5.196	0.631	1.606	0.793	1.596	0.364	0.263	0.759	0.714	1.069	1.412	2.473	-1.731	0.510	51.101
4SSW14	2.510	27.674	5.196	0.738	1.931	0.955	1.618	0.700	0.433	0.974	1.001	1.250	3.303	3.043	-2.386	0.385	51.101

Flange Width: SSCW=1-1/4", SSC=1-3/8", SSJ=1-5/8", SJW=2" and SSW=2-1/2"

Reference Page 10 for Notes

SECTION PROPERTIES

8" - 16"

SSJ, SJW & SSW STUDS

Section	Weight Plf	Ma K-in	Va Kip	Area in ²	Ix in ⁴	Sx _e in ³	Rx in	Iy in ⁴	Sy _e in ³	Ry in	D in	JX1000 in ⁴	Cw in ⁶	Ro in	Xo in	Beta	h/t
8SSJ12	3.848	68.023	11.030	1.131	9.223	2.272	2.856	0.246	0.173	0.466	0.307	3.898	3.218	3.000	-0.792	0.930	73.663
8SJW12	4.237	80.615	11.030	1.245	10.905	2.693	2.959	0.521	0.311	0.647	0.457	4.293	6.661	3.250	-1.178	0.869	73.663
8SSW12	4.778	97.324	11.030	1.404	13.137	3.251	3.059	1.110	0.562	0.889	0.691	4.841	14.658	3.636	-1.753	0.768	73.663
9-1/4SSJ18	2.030	25.485	0.849	0.597	6.672	1.429	3.344	0.170	0.114	0.534	0.324	0.404	2.946	3.501	-0.888	0.936	200.100
9-1/4SSW18	2.174	28.263	0.639	0.639	7.526	1.614	3.432	0.303	0.168	0.689	0.448	0.433	5.199	3.703	-1.208	0.894	200.100
9-1/4SSJ16	2.543	49.139	1.688	0.747	8.319	1.782	3.336	0.212	0.143	0.533	0.330	0.798	3.711	3.495	-0.895	0.934	158.428
9-1/4SSW16	2.760	54.733	1.688	0.811	9.551	2.048	3.432	0.403	0.228	0.705	0.474	0.866	7.090	3.726	-1.267	0.884	158.428
9-1/4SSJ14	3.147	65.089	3.403	0.925	10.151	2.174	3.313	0.243	0.166	0.512	0.318	1.567	4.237	3.459	-0.853	0.939	124.734
9-1/4SSW14	3.420	75.194	3.403	1.005	11.712	2.511	3.414	0.472	0.271	0.685	0.461	1.703	8.229	3.690	-1.223	0.890	124.734
9-1/4SSJ12	4.280	85.031	10.047	1.258	13.272	2.840	3.248	0.253	0.174	0.449	0.281	4.337	4.517	3.359	-0.730	0.953	85.954
9-1/4SSW12	4.670	99.757	10.047	1.372	15.547	3.332	3.366	0.540	0.313	0.627	0.420	4.731	9.295	3.595	-1.095	0.907	85.954
10SSJ16	2.687	52.165	1.558	0.790	10.094	2.003	3.575	0.216	0.143	0.523	0.314	0.843	4.424	3.714	-0.858	0.947	171.678
10SJW16	2.904	58.457	1.558	0.853	11.544	2.293	3.678	0.411	0.228	0.694	0.452	0.911	8.422	3.936	-1.219	0.904	171.678
10SSW16	3.133	62.108	1.558	0.921	13.131	2.611	3.777	0.740	0.334	0.896	0.626	0.983	15.084	4.219	-1.654	0.846	171.678
10SSJ14	3.329	73.245	3.138	0.978	12.329	2.446	3.550	0.247	0.166	0.502	0.303	1.658	5.060	3.677	-0.816	0.951	135.252
10SJW14	3.602	84.246	3.138	1.058	14.166	2.814	3.658	0.481	0.272	0.674	0.440	1.794	9.795	3.901	-1.176	0.909	135.252
10SSW14	3.965	96.031	3.138	1.165	16.549	3.290	3.768	0.952	0.448	0.904	0.646	1.975	19.827	4.231	-1.697	0.839	135.252
10SSJ12	4.540	95.981	9.253	1.334	16.167	3.206	3.481	0.257	0.174	0.439	0.268	4.600	5.417	3.577	-0.698	0.962	93.328
10SJW12	4.929	111.989	9.253	1.449	18.840	3.740	3.606	0.550	0.314	0.616	0.400	4.994	11.122	3.807	-1.051	0.924	93.328
10SSW12	5.470	133.419	9.253	1.607	22.419	4.456	3.735	1.182	0.566	0.858	0.610	5.542	23.993	4.146	-1.585	0.854	93.328
11-1/2SSJ16	2.976	58.517	1.350	0.875	14.323	2.477	4.047	0.222	0.144	0.504	0.286	0.934	6.066	4.154	-0.792	0.964	198.180
11-1/2SSW16	3.422	70.614	1.350	1.006	18.377	3.182	4.275	0.767	0.335	0.874	0.575	1.074	20.531	4.630	-1.548	0.888	198.180
11-1/2SSJ14	3.693	83.505	2.716	1.085	17.526	3.031	4.019	0.253	0.166	0.483	0.277	1.839	6.957	4.117	-0.753	0.967	156.290
11-1/2SSW14	4.329	109.335	2.716	1.272	23.181	4.014	4.268	0.989	0.449	0.882	0.595	2.156	26.910	4.639	-1.588	0.883	156.290
11-1/2SSJ12	5.059	119.570	7.990	1.487	23.105	3.994	3.942	0.264	0.175	0.421	0.246	5.126	7.493	4.016	-0.641	0.975	108.078
11-1/2SSW12	5.989	163.121	7.990	1.760	31.468	5.448	4.228	1.226	0.568	0.835	0.561	6.068	32.829	4.557	-1.480	0.895	108.078
12SSJ16	3.073	60.691	1.292	0.903	15.947	2.645	4.202	0.224	0.144	0.498	0.278	0.964	6.679	4.302	-0.773	0.968	207.014
12SJW16	3.289	68.622	1.292	0.967	18.067	2.998	4.323	0.429	0.229	0.666	0.403	1.032	12.644	4.512	-1.108	0.940	207.014
12SSW16	3.518	73.465	1.292	1.034	20.373	3.382	4.439	0.776	0.335	0.866	0.560	1.104	22.568	4.770	-1.516	0.899	207.014
12SSJ14	3.814	86.019	2.599	1.121	19.524	3.237	4.174	0.255	0.167	0.477	0.269	1.899	7.665	4.265	-0.734	0.970	163.303
12SJW14	4.087	103.953	2.599	1.201	22.208	3.685	4.300	0.501	0.272	0.646	0.392	2.035	14.762	4.477	-1.067	0.943	163.303
12SSW14	4.451	113.355	2.599	1.308	25.704	4.267	4.433	1.000	0.449	0.874	0.580	2.216	29.557	4.779	-1.555	0.894	163.303
12SSJ12	5.232	127.935	7.642	1.538	25.781	4.273	4.095	0.265	0.175	0.416	0.239	5.301	8.268	4.163	-0.624	0.978	112.994
12SJW12	5.621	147.361	7.642	1.652	29.674	4.922	4.238	0.572	0.315	0.588	0.357	5.695	16.927	4.383	-0.950	0.953	112.994
12SSW12	6.162	173.525	7.642	1.811	34.917	5.796	4.391	1.239	0.568	0.827	0.547	6.243	36.137	4.697	-1.449	0.905	112.994
14SJW16	3.675	79.140	1.104	1.080	26.524	3.331	4.956	0.443	0.229	0.641	0.363	1.153	17.868	5.100	-1.017	0.960	242.350
14SJW14	4.572	113.246	2.218	1.344	32.654	4.650	4.930	0.517	0.273	0.621	0.354	2.277	20.915	5.064	-0.978	0.963	191.353
14SSW14	4.936	129.723	2.218	1.451	37.477	5.339	5.083	1.038	0.450	0.846	0.526	2.458	41.626	5.349	-1.437	0.928	191.353
14SJW12	6.313	186.758	6.509	1.855	43.813	6.238	4.859	0.589	0.316	0.563	0.324	6.397	24.130	4.968	-0.868	0.970	132.660
14SSW12	6.854	217.663	6.509	2.014	51.038	7.270	5.034	1.284	0.570	0.798	0.497	6.944	51.238	5.269	-1.335	0.936	132.660
16SSW14	5.421	146.438	1.935	1.593	52.152	6.192	5.721	1.069	0.450	0.819	0.482	2.700	56.124	5.932	-1.336	0.949	219.404
16SSW12	7.546	251.869	5.669	2.218	71.191	8.879	5.666	1.321	0.571	0.772	0.456	7.646	69.405	5.851	-1.240	0.955	152.325

Flange Width: SSJ=1-5/8", SJW=2" and SSW=2-1/2"

Reference Page 10 for Notes

SUGGESTED DESIGN GUIDES

ALLOWABLE AXIAL LOADS

Braced Studs Subjected to Lateral Load

P_{allow} , KIPS (1,000 Lbs) Per STUD

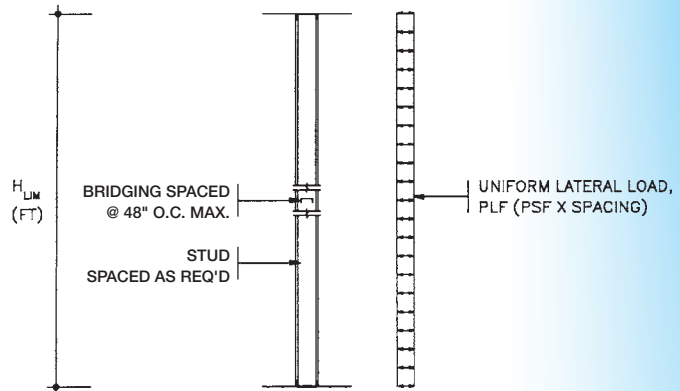
Use:

Tables allow for the selection of an axially loaded stud in the absence or presence of bending due to lateral load. The values assume the studs are typically bridged at intervals not to exceed 48" on center with alternative labor saving values published for 9 and 10 foot high walls where a single row of bridging at mid-height of the wall would suffice.

Select a stud, based on applied lateral load, spacing and height providing an allowable axial load in excess of the applied load.

Notes:

1. The tabulated values represent the least allowable axial load of the stud in the presence of lateral load or in the absence of lateral load.
2. Values shown are limited to studs used in a simply supported application. Conditions involving cantilevers, concentrated loads, eccentricities, multiple spans, etc. should be investigated separately. The values assume the axial load is applied concentrically to the member.
3. Values are based on the installation of mechanical bridging spaced at intervals not to exceed 48" on center (columns denoted 48) with the exception of the columns entitled 9FT/MID and 10FT/MID where a single row of bridging may be installed at mid-height of the wall.
4. Stud ends shall be restrained against rotation by means of a fixed attachment to each side of continuous track.



5. Deflections due to lateral load do not exceed $L/720$ unless noted as follows:
 - a. values followed by the subscript 6 (x.xx6) do not exceed $L/600$
 - b. values followed by the subscript 3 (x.xx3) do not exceed $L/360$
 - c. values followed by the subscript 2 (x.xx2) do not exceed $L/240$
 - d. values have been omitted where deflection exceeds $L/240$

Lateral load deflections were calculated without regard to the composite contribution of collateral materials.
6. Contact Super Stud for axial load capacities of sections not shown herein.

Note: All tables based on $F_y=50$ KSI for 12, 14, 16 gage material, $F_y=33$ KSI for 18 & 20 gage material.



SUGGESTED DESIGN GUIDES

ALLOWABLE UNIFORM LOAD CAPACITIES HEADERS

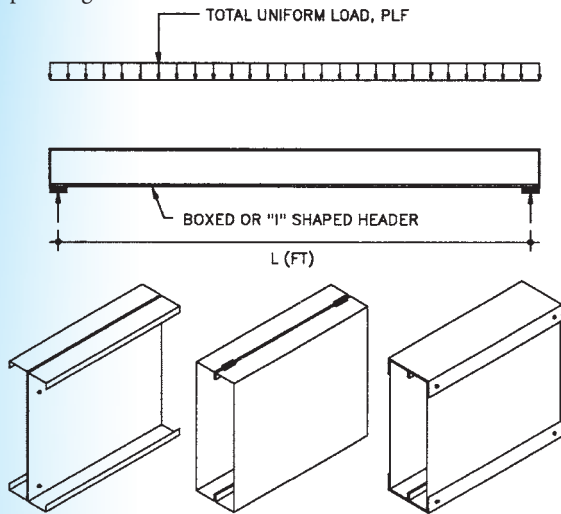
Pounds Per Lineal Foot, PLF

Use:

The tables are used to select a simply supported boxed or "I" shaped header subjected to uniform load. Select a header which provides an allowable uniform load in excess of the applied load.

Notes:

1. The values represent the allowable total load, in pounds per lineal foot (PLF), limited by the bending or shear capacity of the header. Additionally, deflection was limited to 1/360 of span length.



SECTION	4FT	5FT	6FT	7FT	8FT	9FT	10FT	11FT	12FT	13FT	14FT	15FT	16FT
(2)4SSJ18	730	467	324	229	154	108	79	59	45	36	29	23	19
(2)4SSJ16	1369	778	450	284	190	133	97	73	56	44	35	29	24
(2)4SSJ14	1654	941	545	343	230	161	118	88	68	54	43	35	29
(2)6SSJ18	1275	816	567	416	319	252	204	153	118	93	74	61	50
(2)6SSJ16	2401	1537	1067	740	496	348	254	191	147	116	93	75	62
(2)6SSJ14	2915	1865	1295	898	602	423	308	232	178	140	112	91	75
(2)8SSJ18	985	788	657	563	475	375	304	251	211	180	149	121	100
(2)8SSJ16	1962	1569	1308	1121	896	698	509	382	295	232	185	151	124
(2)8SSJ14	3959	2793	1940	1425	1091	850	620	466	359	282	226	184	151
(2)8SSJ12	5669	3628	2519	1851	1417	1106	806	606	467	367	294	239	197

2. The use of these tables is limited to simply supported conditions. Applications involving cantilevers, concentrated loads, eccentricities, multiple spans, impact loading, etc. should be investigated separately.
3. The compression flange of the header shall be laterally braced at intervals not to exceed 2'-0" on center.
4. Web crippling should be investigated separately. Web reinforcement is suggested at all bearing and/or concentrated load locations unless deemed unnecessary by analysis.

Shapes having multiple un-reinforced webs (sections which provide a high degree of restraint against rotation of the web) subjected to a combination of concentrated load or reaction and bending shall be designed to meet the requirements of AISI Section C3.4. Reference Page 29 for additional information.

Avoid locating a web knockout within a distance equalling 1.5 x depth of the member (h) from the edge of bearing. Should a knockout be located in this area, web reinforcement is required.

5. Contact Super Stud for allowable load capacities of sections not shown herein.

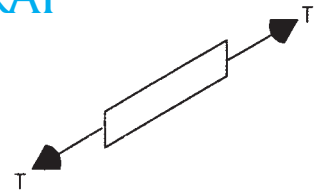
SECTION	8FT	9FT	10FT	11FT	12FT	13FT	14FT	15FT	16FT	17FT	18FT	19FT	20FT
(2)6SSJ18										42	35	30	26
(2)6SSJ16										52	44	37	32
(2)6SSJ14										63	53	45	39
(2)8SSJ18										83	70	60	51
(2)8SSJ16										104	87	74	64
(2)8SSJ14										126	106	90	77
(2)8SSJ12										164	138	118	101
(2)10SSJ16	779	692	623	567	483	402	322	261	215	180	151	129	110
(2)10SSJ14	1526	1206	977	807	624	491	393	319	263	219	185	157	135
(2)10SSJ12	2000	1580	1280	1058	818	643	515	419	345	288	242	206	177
(2)12SSJ16	646	574	517	470	431	398	369	345	316	280	239	203	174
(2)12SSJ14	1300	1142	832	625	482	379	303	247	203	169	143	121	104
(2)12SSJ12	2665	2106	1706	1410	1185	1009	821	668	550	459	386	329	282
(2)14SJW14	1109	986	887	807	739	682	634	591	555	522	466	416	357
(2)14SJW12	3255	2893	2490	2058	1729	1473	1270	1107	935	779	657	558	479

ALLOWABLE TENSILE CAPACITIES STRAP

T_{ALLOW}, KIPS (1,000 LBS) PER STRAP

Use:

The table provides the allowable capacity of strap subjected to tensile forces.



T _{allow} , KIPS (1,000 LBS)	
2" FS 20 Gage	1.37
3" FS 20 Gage	2.05
2" FS 18 Gage	1.78
3" FS 18 Gage	2.67
4" FS 18 Gage	3.57

T _{allow} , KIPS (1,000 LBS)	
2" FS 16 Gage	2.24
3" FS 16 Gage	3.36
4" FS 16 Gage	4.48
3" FS 14 Gage	6.41
4" FS 14 Gage	8.55
5" FS 14 Gage	10.69

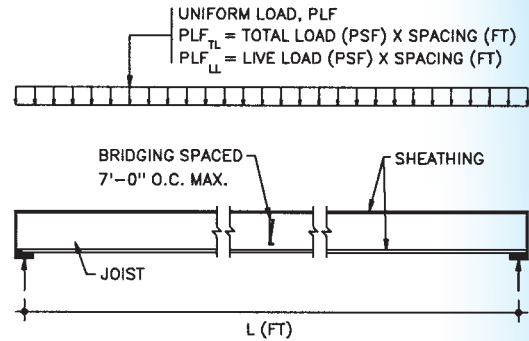
SUGGESTED DESIGN GUIDES

ALLOWABLE UNIFORM LOAD CAPACITIES JOISTS

Pounds Per Lineal Foot, PLF

Use:

The tables are used to select a simply supported joist or rafter subjected to uniform lateral loads. Select a joist, based on span and deflection limit, which provides an allowable uniform load in excess of the applied load.



- Joist ends shall be restrained against rotation by means of a fixed attachment to each side of continuous track or alternative methods preventing end rotation shall be provided.
- Deflections were calculated without regard to the composite contribution of collateral materials.
- Web crippling per AISI Section C3.4 should be investigated separately. Web reinforcement is suggested at all bearing and/or concentrated load locations unless deemed unnecessary by analysis. Reference Pages 29 and 30 for allowable un-reinforced web capacities.
- Contact Super Stud for allowable load capacities of sections not shown herein.

Notes:

- TL values denote the TOTAL LOAD capacity of the section expressed in pounds per lineal foot (PLF), which will not exceed stress limitations or generate deflections in excess of $L/240$. LL values denote the LIVE LOAD, in Pounds per Lineal Foot (PLF), which will generate a deflection equalling $L/360$.

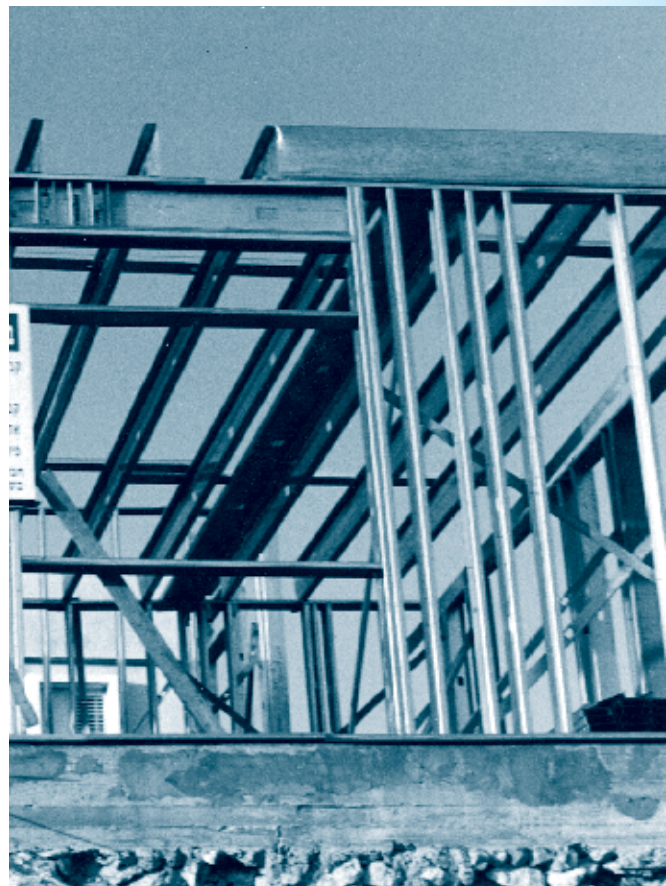
To determine LIVE LOAD deflection limits of $L/480$ and $L/240$, multiply the LL values 0.75 and 1.5 respectively. In either case, the TOTAL LOAD capacity of the joist shall not be exceeded.

- To determine the equivalent Pound per Square Foot (PSF) load carrying capacities, divide the Pound per Lineal Foot (PLF) values by the joist spacing expressed in feet.

$$\text{Pound Per Square Foot (PSF)} = \frac{\text{Pound Per Lineal Foot (PLF)}}{\text{Joist Spacing (Feet On Center)}}$$

For joists spaced: 12" O.C., divide values by 1.0
 16" O.C., divide values by 1.333
 24" O.C., divide values by 2.0

- The use of these tables is limited to simply supported conditions installed to a maximum slope of $1/2"$ per foot. Conditions involving cantilevers, concentrated loads, eccentricities, multiple spans, impact loading, etc. should be investigated separately.
- The compression flange of the section should be braced by means of the attachment of continuous diaphragm rated sheathing. Additionally, mechanical bridging shall be installed at intervals not to exceed 7'-0" on center.



SUGGESTED DESIGN GUIDES

ALLOWABLE CAPACITIES

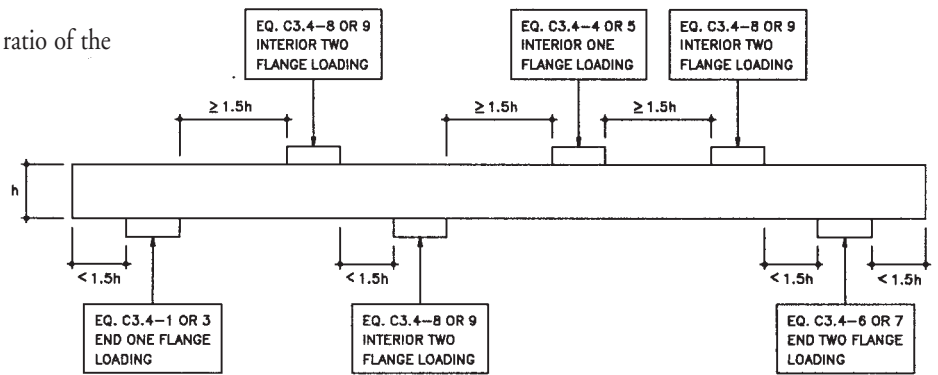
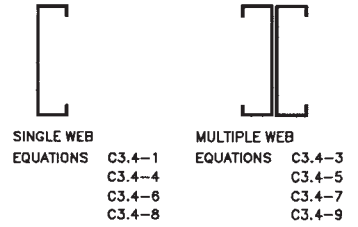
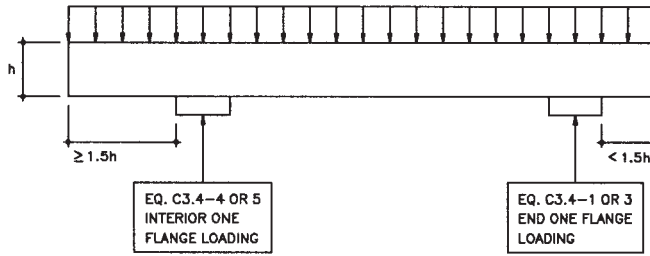
Unstiffened Web(s) Subjected to Local Forces,
 P_{allow} , KIPS (1,000 Lbs) Per Web(s)

Use:

The tables are used to verify the allowable capacity of an unreinforced web subjected to local forces.

Notes:

- Values have been omitted where the height to thickness ratio of the web, h/t , exceeds 200.
- Values shown for equations C3.4-1, 3.4-4, 3.4-6 and 3.4-8 represent the allowable concentrated load or reaction for one solid unreinforced web. Values shown for equations C3.4-3, 3.4-5, 3.4-7 and 3.4-9 represent the allowable concentrated load or reaction for "I" shaped sections connected back to back or similar sections which provide a high degree of restraint against rotation of the web.
- Unreinforced flat webs of sections subjected to a combination of concentrated load or reaction and bending shall be designed to meet the following requirements:
 - For shapes having single unreinforced webs: $1.2(P/P_a) + (M/M_a) < 1.5$
 - For shapes having multiple unreinforced webs (i.e. "I" sections or similar sections which provide a high degree of restraint against rotation of the web):



$$1.1(P/P_a) + (M/M_a) < 1.5$$

Where

P = Concentrated load or reaction

P_a = Allowable concentrated load or reaction

M = Applied bending moment at, or immediately adjacent to, the point of application of the concentrated load or reaction

M_a = Allowable bending moment if bending alone exists

- Avoid locating a web knockout within a distance equalling 1.5 x depth of the member (h) from the edge of bearing. Should a knockout be located in this area, web reinforcement is required.

SECTION	3-5/8"X20GASTUD				3-5/8"X18GASTUD				3-5/8"X16GASTUD				3-5/8"X14GASTUD				3-5/8"X12GASTUD			
	1.5	2.5	3.5	4.5	1.5	2.5	3.5	4.5	1.5	2.5	3.5	4.5	1.5	2.5	3.5	4.5	1.5	2.5	3.5	4.5
EQ.C3.4-1	.221	.265	.310	.354	.386	.451	.515	.579	.752	.857	.962	1.067	1.173	1.309	1.445	1.581	2.332	2.531	2.731	2.931
EQ.C3.4-4	.373	.442	.533	.624	.634	.714	.825	.951	1.323	1.461	1.596	1.813	2.072	2.250	2.427	2.609	4.145	4.403	4.662	4.921
EQ.C3.4-6	.153	.184	.214	.245	.272	.318	.363	.408	.536	.611	.686	.761	.843	.941	1.038	1.136	1.691	1.836	1.981	2.126
EQ.C3.4-8	.356	.369	.382	.394	.669	.688	.706	.724	1.490	1.523	1.556	1.589	2.457	2.501	2.545	2.588	5.215	5.280	5.345	5.411

SECTION	4"X20GASTUD				4"X18GASTUD				4"X16GASTUD				4"X14GASTUD				4"X12GASTUD			
	1.5	2.5	3.5	4.5	1.5	2.5	3.5	4.5	1.5	2.5	3.5	4.5	1.5	2.5	3.5	4.5	1.5	2.5	3.5	4.5
EQ.C3.4-1	.215	.259	.302	.345	.380	.443	.506	.569	.742	.845	.949	1.052	1.161	1.295	1.429	1.564	2.315	2.513	2.712	2.910
EQ.C3.4-4	.367	.435	.524	.614	.626	.705	.815	.939	1.310	1.447	1.581	1.795	2.056	2.232	2.408	2.589	4.123	4.380	4.637	4.895
EQ.C3.4-6	.148	.177	.207	.237	.266	.310	.354	.399	.526	.600	.673	.747	.831	.928	1.024	1.120	1.675	1.819	1.963	2.106
EQ.C3.4-8	.340	.352	.365	.377	.648	.666	.684	.702	1.455	1.487	1.519	1.552	2.414	2.457	2.499	2.542	5.153	5.217	5.282	5.347

SUGGESTED DESIGN GUIDES

ALLOWABLE CAPACITIES

Unstiffened Web(s) Subjected to Local Forces,

P_{allow} , KIPS (1,000 Lbs) Per Web(s)

SECTION BEARING (IN)	6"X20 GASTUD				6"X18 GASTUD				6"X16 GASTUD				6"X14 GASTUD				6"X12 GASTUD			
	1.5	2.5	3.5	4.5	1.5	2.5	3.5	4.5	1.5	2.5	3.5	4.5	1.5	2.5	3.5	4.5	1.5	2.5	3.5	4.5
EQ.C3.4-1	.187	.224	.262	.299	.343	.400	.457	.514	.687	.783	.879	.974	1.094	1.221	1.348	1.475	2.225	2.416	2.607	2.797
EQ.C3.4-4	.332	.394	.475	.556	.583	.657	.759	.874	1.240	1.370	1.496	1.700	1.971	2.140	2.308	2.481	4.006	4.256	4.506	4.756
EQ.C3.4-6	.121	.145	.169	.194	.231	.270	.308	.347	.475	.541	.607	.674	.769	.858	.947	1.037	1.591	1.728	1.864	2.000
EQ.C3.4-8	.255	.265	.274	.283	.537	.552	.566	.581	1.269	1.297	1.325	1.353	2.181	2.220	2.258	2.297	4.823	4.884	4.944	5.005
EQ.C3.4-3	.867	.982	1.075	1.155	1.357	1.523	1.658	1.775	2.996	3.339	3.618	3.859	4.434	4.907	5.291	5.623	8.226	9.007	9.642	10.190
EQ.C3.4-5	1.294	1.515	1.695	1.851	2.074	2.410	2.682	2.918	3.394	3.916	4.340	4.706	5.199	5.955	6.568	7.099	10.194	11.544	12.641	13.590
EQ.C3.4-7	.445	.504	.551	.593	.797	.894	.974	1.042	1.315	1.465	1.588	1.694	2.198	2.433	2.623	2.788	4.928	5.395	5.776	6.104
EQ.C3.4-9	1.086	1.272	1.423	1.554	1.874	2.177	2.424	2.637	2.973	3.430	3.801	4.122	4.745	5.434	5.995	6.479	9.613	10.885	11.920	12.814

SECTION BEARING (IN)	8"X18 GASTUD				8"X16 GASTUD				8"X14 GASTUD				8"X12 GASTUD			
	1.5	2.5	3.5	4.5	1.5	2.5	3.5	4.5	1.5	2.5	3.5	4.5	1.5	2.5	3.5	4.5
EQ.C3.4-1	.306	.357	.408	.459	.632	.720	.808	.896	1.028	1.147	1.266	1.385	2.136	2.319	2.502	2.685
EQ.C3.4-4	.540	.608	.703	.809	1.170	1.292	1.412	1.604	1.886	2.047	2.209	2.374	3.889	4.132	4.374	4.617
EQ.C3.4-6	.197	.230	.263	.295	.423	.482	.541	.600	.707	.789	.871	.953	1.507	1.636	1.765	1.895
EQ.C3.4-8	.425	.437	.449	.461	1.082	1.107	1.131	1.155	1.948	1.983	2.017	2.052	4.494	4.550	4.606	4.663
EQ.C3.4-3	1.391	1.561	1.700	1.819	3.121	3.478	3.769	4.020	4.584	5.073	5.470	5.813	8.428	9.227	9.877	10.439
EQ.C3.4-5	2.074	2.410	2.682	2.918	3.394	3.916	4.340	4.706	5.199	5.955	6.568	7.099	10.194	11.544	12.641	13.590
EQ.C3.4-7	.748	.839	.914	.978	1.253	1.396	1.513	1.613	2.118	2.344	2.527	2.686	4.806	5.262	5.632	5.953
EQ.C3.4-9	1.736	2.017	2.246	2.443	2.806	3.237	3.588	3.891	4.541	5.201	5.737	6.200	9.509	10.768	11.791	12.676

SECTION BEARING (IN)	9-1/4"X16 GASTUD				9-1/4"X14 GASTUD				9-1/4"X12 GASTUD			
	1.5	2.5	3.5	4.5	1.5	2.5	3.5	4.5	1.5	2.5	3.5	4.5
EQ.C3.4-1	.597	.681	.764	.848	.987	1.101	1.215	1.330	2.080	2.258	2.436	2.614
EQ.C3.4-4	1.127	1.244	1.359	1.544	1.832	1.989	2.146	2.307	3.816	4.054	4.292	4.530
EQ.C3.4-6	.391	.445	.500	.554	.668	.745	.823	.900	1.455	1.579	1.704	1.828
EQ.C3.4-8	.966	.988	1.009	1.030	1.802	1.834	1.866	1.898	4.288	4.341	4.395	4.449

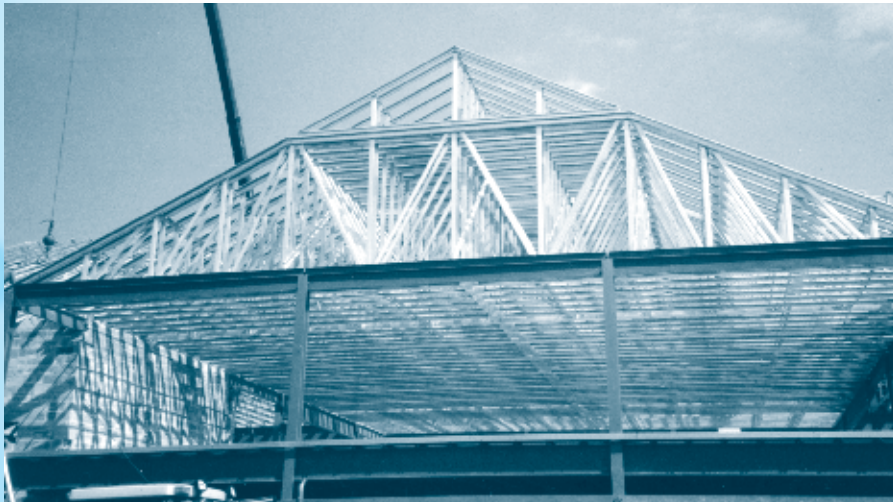
SECTION BEARING (IN)	10"X16 GASTUD				10"X14 GASTUD				10"X12 GASTUD			
	1.5	2.5	3.5	4.5	1.5	2.5	3.5	4.5	1.5	2.5	3.5	4.5
EQ.C3.4-1	.577	.657	.738	.818	.962	1.073	1.185	1.296	2.046	2.221	2.397	2.572
EQ.C3.4-4	1.100	1.215	1.327	1.508	1.801	1.955	2.109	2.267	3.772	4.007	4.243	4.478
EQ.C3.4-6	.371	.423	.475	.527	.645	.719	.794	.869	1.423	1.545	1.667	1.789
EQ.C3.4-8	.896	.916	.936	.956	1.715	1.745	1.776	1.806	4.164	4.216	4.269	4.321
EQ.C3.4-3	3.169	3.532	3.827	4.082	4.734	5.239	5.649	6.003	8.629	9.448	10.113	10.689
EQ.C3.4-5	3.394	3.916	4.340	4.706	5.199	5.955	6.568	7.099	10.194	11.544	12.641	13.590
EQ.C3.4-7	1.190	1.327	1.438	1.533	2.038	2.255	2.432	2.584	4.684	5.128	5.489	5.802
EQ.C3.4-9	2.640	3.045	3.375	3.660	4.337	4.967	5.479	5.922	9.225	10.446	11.439	12.297

SECTION BEARING (IN)	11-1/4"X16 GASTUD				11-1/4"X14 GASTUD				11-1/4"X12 GASTUD			
	1.5	2.5	3.5	4.5	1.5	2.5	3.5	4.5	1.5	2.5	3.5	4.5
EQ.C3.4-1	.536	.610	.685	.760	.912	1.018	1.123	1.229	1.979	2.148	2.318	2.488
EQ.C3.4-4	1.048	1.157	1.264	1.436	1.737	1.885	2.034	2.186	3.684	3.914	4.144	4.374
EQ.C3.4-6	.333	.379	.426	.472	.598	.667	.737	.806	1.360	1.476	1.593	1.709
EQ.C3.4-8	.757	.773	.790	.807	1.540	1.568	1.595	1.622	3.917	3.966	4.015	4.064

SECTION BEARING (IN)	12"X14 GASTUD				12"X12 GASTUD				14"X14 GASTUD				14"X12 GASTUD				16"X12 GASTUD			
	1.5	2.5	3.5	4.5	1.5	2.5	3.5	4.5	1.5	2.5	3.5	4.5	1.5	2.5	3.5	4.5	1.5	2.5	3.5	4.5
EQ.C3.4-1	.896	.999	1.103	1.207	1.956	2.124	2.292	2.459	.829	.925	1.021	1.118	1.867	2.027	2.187	2.347	1.777	1.930	2.082	2.234
EQ.C3.4-4	1.715	1.862	2.009	2.159	3.655	3.883	4.111	4.339	1.630	1.769	1.909	2.052	3.538	3.759	3.980	4.200	3.421	3.635	3.848	4.061
EQ.C3.4-6	.582	.650	.717	.785	1.339	1.454	1.568	1.683	.520	.581	.641	.701	1.255	1.362	1.470	1.577	1.170	1.271	1.371	1.471
EQ.C3.4-8	1.482	1.508	1.535	1.561	3.835	3.883	3.931	3.979	1.249	1.271	1.294	1.316	3.505	3.549	3.593	3.637	3.175	3.215	3.255	3.295
EQ.C3.4-3	4.813	5.326	5.743	6.103	8.830	9.668	10.349	10.938	4.813	5.326	5.743	6.103	9.031	9.888	10.585	11.187	9.209	10.083	10.793	11.407
EQ.C3.4-5	5.199	5.955	6.568	7.099	10.194	11.544	12.641	13.590	5.199	5.955	6.568	7.099	10.194	11.544	12.641	13.590	10.194	11.544	12.641	13.590
EQ.C3.4-7	1.958	2.166	2.336	2.483	4.562	4.994	5.346	5.650	1.877	2.078	2.240	2.381	4.439	4.861	5.203	5.499	4.317	4.727	5.060	5.348
EQ.C3.4-9	4.133	4.733	5.221	5.643	8.941	10.124	11.086	11.918	3.929	4.500	4.963	5.365	8.656	9.802	10.734	11.539	8.372	9.480	10.381	11.160

Roof Systems

Super Stud can furnish engineered roof systems. Rafters and joists that are designed & engineered for your demanding project, precut, ready to assemble, that save time and money.



Trusses

Ready to assemble custom made trusses. Our trusses are designed to have superior strength while minimizing the weight of the stud.

Pre-cut ready to assemble trusses

This system allows for material to get to your project quicker. These trusses can be assembled quickly on site & installed efficiently meeting today's difficult project deadlines.



SUGGESTED FRAMING APPLICATIONS

Forward

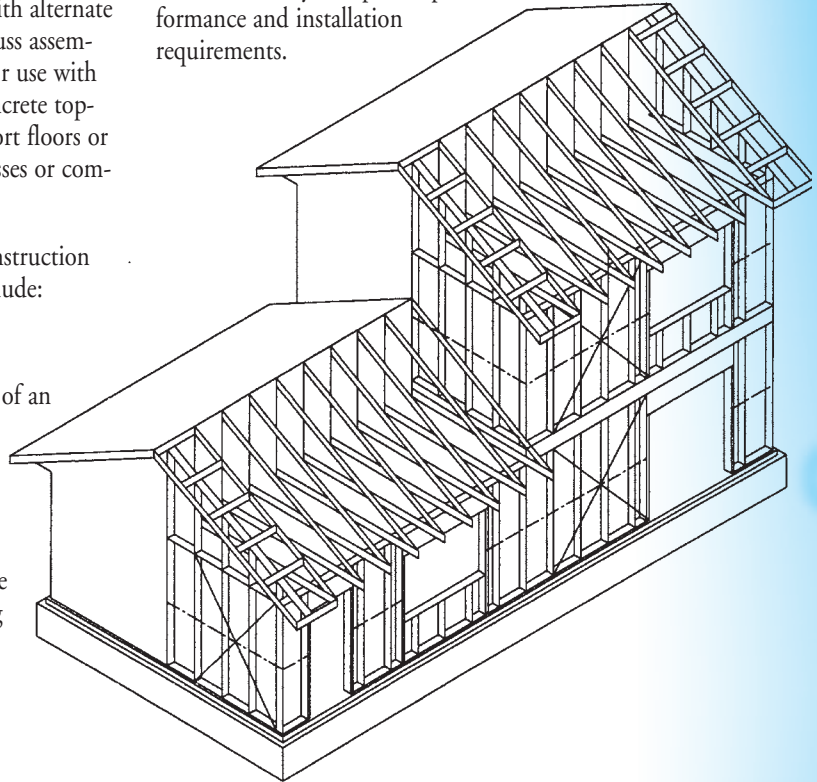
Steel framing may be used in the construction of roof, floor and wall assemblies, either collectively or in conjunction with alternate systems. Roofing applications include flat, rafter or truss assemblies. In floor construction, steel framing is suitable for use with either plywood sub-flooring or metal decking with concrete toppings. Fire rated wall assemblies may be used to support floors or roofs constructed with steel or wood joists, rafters, trusses or composite concrete decks.

The framing conditions shown herein are representative of typical construction practices. The user is cautioned to thoroughly evaluate the assembly for specific performance and installation requirements.

Additionally, steel framing may be utilized in the reconstruction and renovation of existing buildings. Applications include:

- Flat to slope roof conversions
- Modification of interior spaces (i.e. the conversion of an interior warehouse to office or storage areas)
- Wood to steel joist replacements
- Fascia modifications to commercial centers

Steel framing may be prefabricated either at the job site or shipped to the project from remote facilities helping to reduce construction time. Pre-fabricated frames manufactured in a controlled environment are built with uniform consistency.



1. BEARING WALL APPLICATIONS

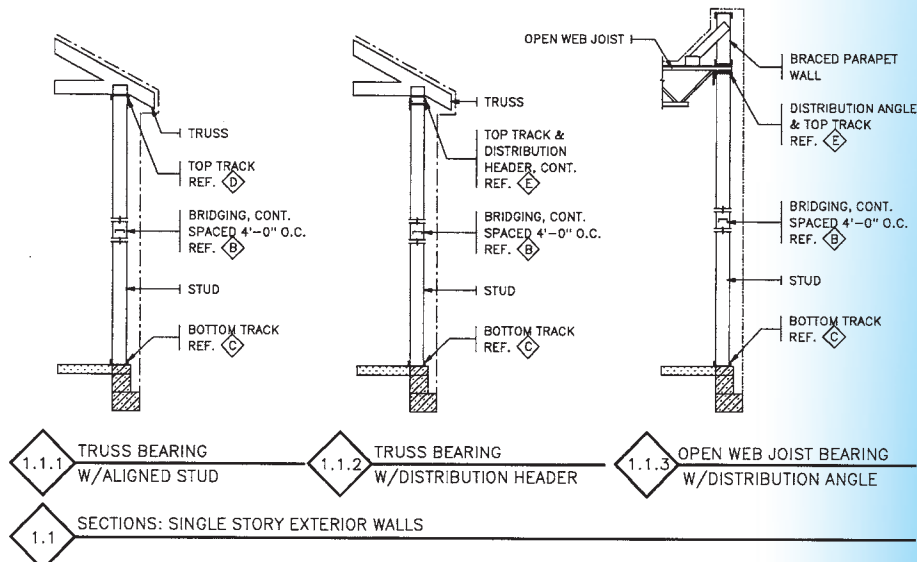
1.1 Single Story Exterior Walls

The wall functions to withstand axial stresses due to roof and floor loads acting in conjunction with bending stresses due to lateral loads. Two bearing conditions are shown. Section 1.1.1 shows the roof element aligning with the stud.

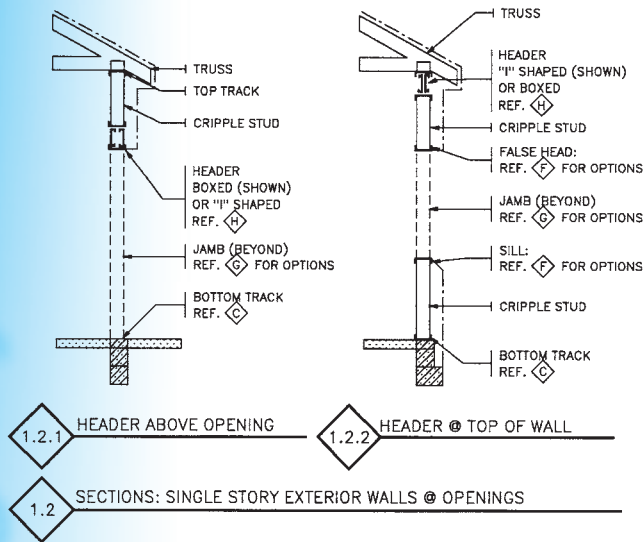
Drawings 1.1.2 & 1.1.3 show the installation of a continuous distribution header at conditions of non-alignment.

The distribution header should maintain a low profile (i.e. avoid "tall" boxed headers) to alleviate overturning or rolling of the header when the wall is subjected to lateral load.

In applications involving open web steel joists, a continuous structural steel angle serves to function as a distribution header while it provides a compatible surface to allow for the welded attachment of the joist to the top of the wall.



SUGGESTED FRAMING APPLICATIONS



1.2 Single Story Exterior Walls at Openings

Two acceptable methods of installing headers over wall openings are shown. Section 1.2.1 shows the header located above the opening while Section 1.2.2 locates the header at the top of the wall. Headers at the top of the wall eliminate the need to "seat" the cripple studs as the load is carried directly to the jamps.

Both boxed or "I" shaped headers are acceptable and may be used interchangeably in these drawings. Boxed headers offer greater torsional restraint while the "I" shaped header is easier to insulate and offers increased resistance to web crippling.

Similar to curtainwall design, each element of the opening (head, sill and jamps) shall provide the necessary stiffness to assure the performance of the exterior wall system.

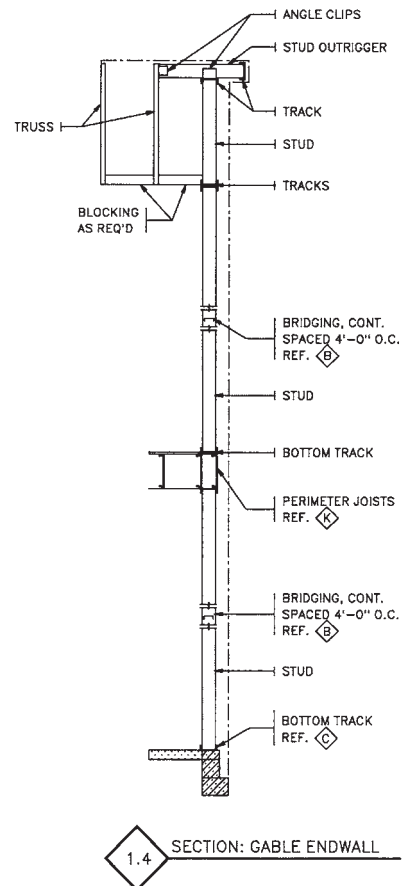
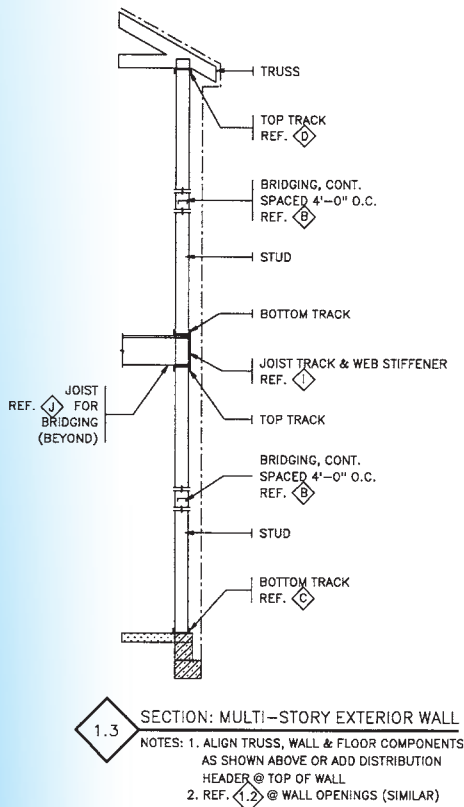
1.3 Multi-story Exterior Walls

Consideration shall be given to the accumulative loads descending the assembly. The joist ends are typically reinforced to prevent web crippling.

Reference Section 1.2 for discussion of framing requirements at wall openings.

1.4 Gable End Walls

The drawing shows the installation of a separate gable infill wall above the top level wall which requires bracing to the roof assembly by means of blocking (shown) or diagonal braces within the roof cavity. As an alternate, the studs may be extended full height to follow the rake of the roof thus eliminating the break in the wall.

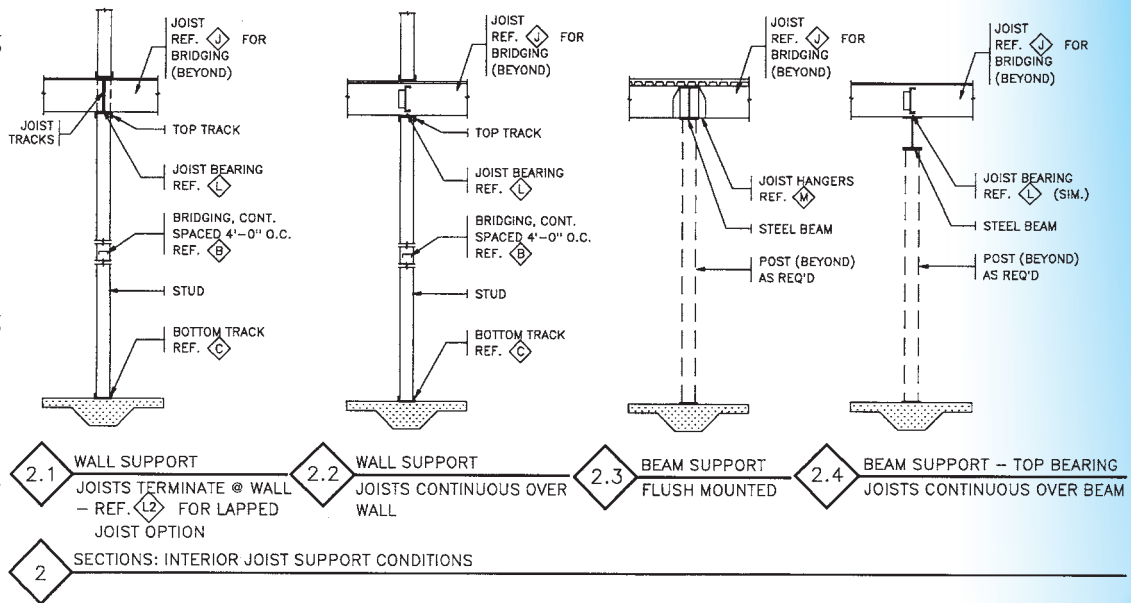


SUGGESTED FRAMING APPLICATIONS

2. INTERIOR JOIST SUPPORT CONDITIONS

Options for providing intermediate joist support are shown involving stud bearing walls (Ref. 2.1 & 2.2) and steel beams (Ref. 2.3 & 2.4).

Section 2.1 shows the joists terminating at continuous tracks centered over the wall. Detail L2, Page 37, describes a lapped joist alternate. Section 2.2 highlights the installation of a continuous joist over a bearing wall.

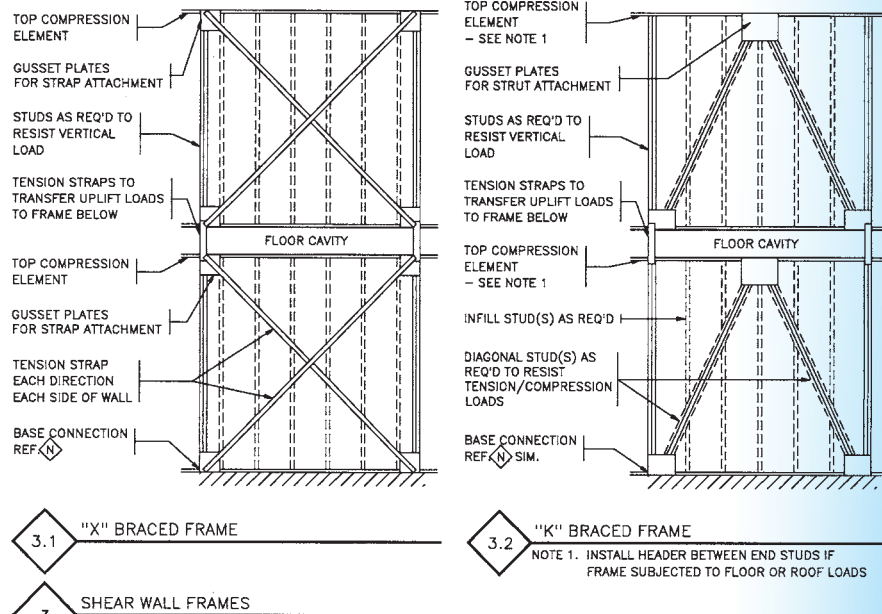


The steel beam alternatives are distinguished by a flush (Ref. 2.3) or top bearing condition (Ref. 2.4). For flush mounted installations, a Super Stud Joist Hanger should be specified.

3. Lateral Frame Stability

The design of lateral bracing for a cold formed steel structure, particularly multi-story frames, is a complex task and as such, a thorough explanation is beyond the scope of this publication. Among items to consider, however, include:

- Determination of dynamic forces due to wind or seismic action;
- Distribution of forces to roof and floor diaphragms to vertical shear walls which are ultimately anchored to the foundation;
- Relative stiffness of the frames and restriction of story drift.



Shear resistance may be provided by means of diaphragm rated structural sheathing attached to each face of a stud partition, "X" braced flat strap assemblies (Ref. 3.1) or "K" braced walls involving the use of diagonal stud components to act as compression and tension elements (Ref. 3.2).

In "X" braced assemblies, the strap thicknesses should be kept to a minimum (i.e. use wider straps of thinner materials) and preferably the straps should cross between stud spacings. These measures help maintain a low profile across the face of the wall to assure a smooth appearance of finished gypsum wall boards. Additionally, the strap should be pulled taut before final attachments are made.

The "K" braced wall, as shown, is typical of a frame found at a non-load bearing partition (i.e. the floor and roof components par-

allel the wall). Should the assembly serve as a bearing partition, a header should be added at the top of the wall continuous between the end studs of the frame. Additionally, the diagonals provide lateral support for the infill studs and its capacity to serve as a "wind girt" should also be reviewed.

The shear wall may develop uplift forces between descending floors and at its base connection to the foundation. Reducing the angle of incidence of the diagonals components will help to decrease uplift forces. Where possible, diagonals should terminate at load bearing post or jamb locations thus taking advantage of counteracting dead loads.

SUGGESTED FRAMING APPLICATIONS

4. Roof Framing

Steel framing is compatible for use in virtually any short to mid-span roofing application involving varying degrees of pitch and support conditions.

Pitched roofs are generally defined by one of two methods: Common Rafters and Ceiling Joists (Ref. 4.1) and Trusses (Ref. 4.2). Flat or low pitched roofs parallel the design and installation requirements of floor joists (Ref. Section 2.).

4.1 Rafter and Ceiling Joist Method

This method of roof construction is known by most carpenters familiar with conventional wood framing.

The rafters terminate at common ridge and hip and valley collectors, which in addition to the perimeter walls, support the rafter. The collectors may be constructed using light gage steel framing sections fabricated into boxed assemblies or structural steel tube or beam shapes.

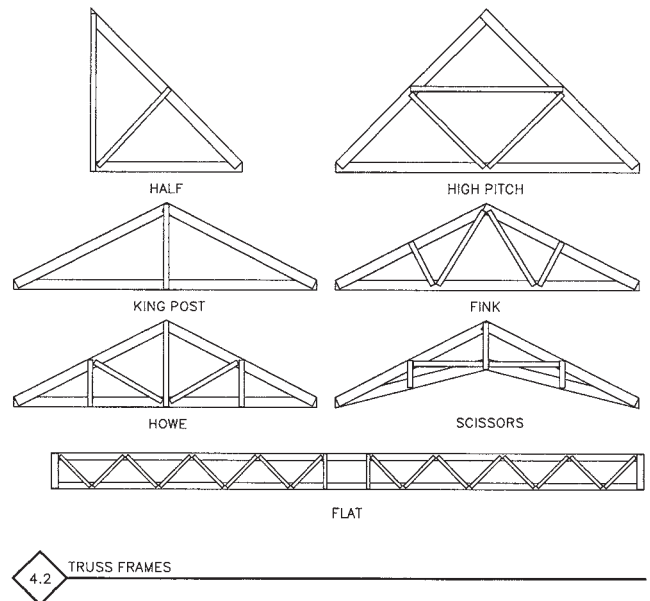
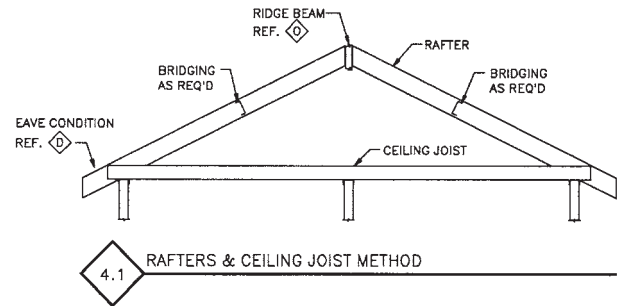
The ceiling joists act as a collar tie between the exterior walls while they serve as joists for an attic or floor.

4.2 Trussed Frame Construction

Compared to rafter and joist construction, prefabricated trusses save materials and reduce on-site labor costs. They are capable of spanning greater distances thus reducing or eliminating interior support locations and increasing space design flexibility.

The design of a truss frame shall consider roof live or snow loads, component dead weights and wind. The slope of the roof need also be considered. Generally, the shallower the slope, the greater the stresses thus requiring increased member sizes, connections, etc.

Truss spacing should also be considered. In applications involving the direct attachment of plywood sheathing, spacing is typically limited to 24 inches on center maximum. Installations involving seamed metals, on the other hand, may allow truss spacings ranging between 4 to 8 feet on center depending on the type and size of the sub-purlin used.



Super Stud has the capability to furnish prefabricated trusses custom manufactured to meet specific project requirements. Contact your local Super Stud sales representative at 718-545-7500 for additional information.

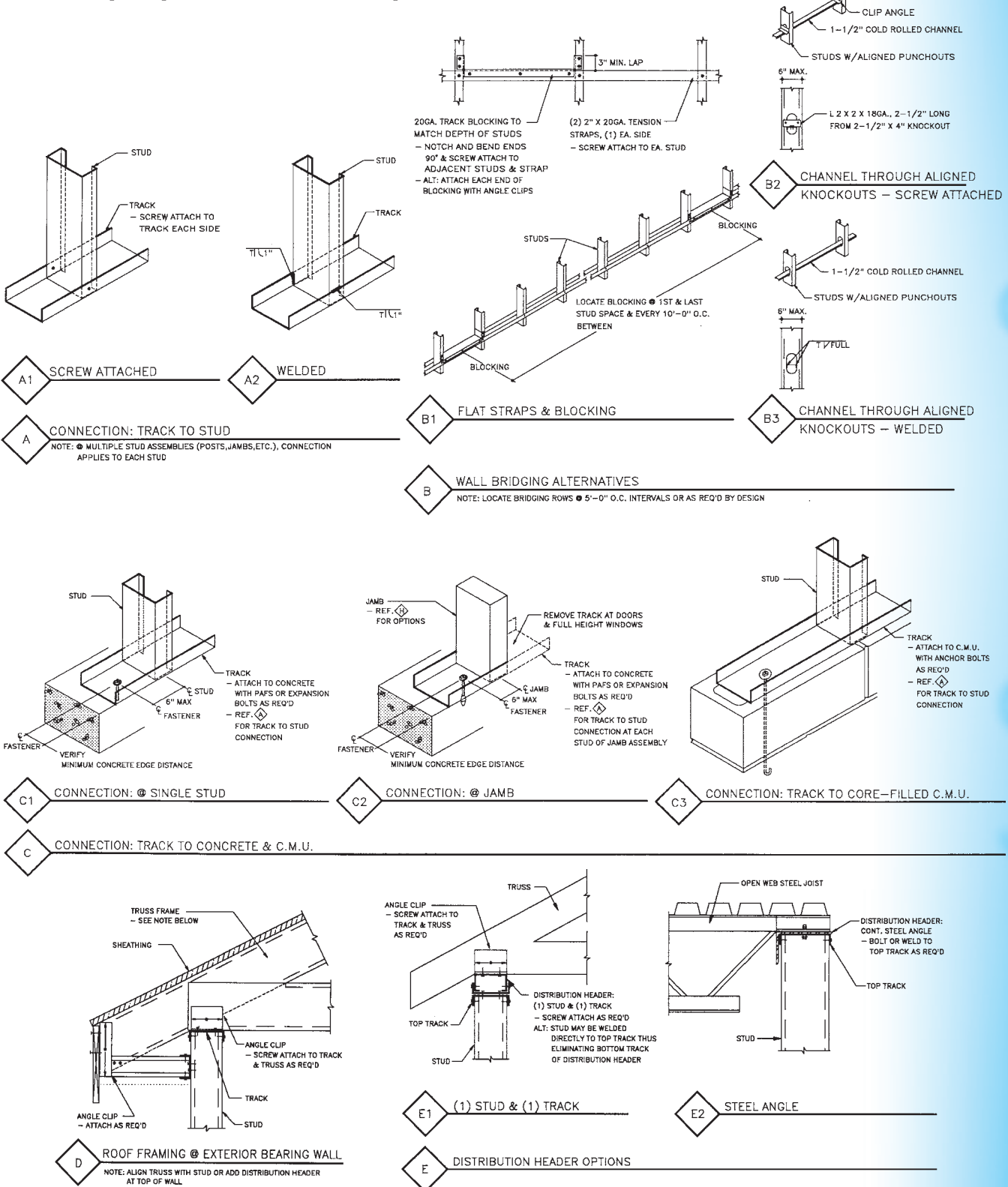


SUGGESTED CONNECTION DETAILS

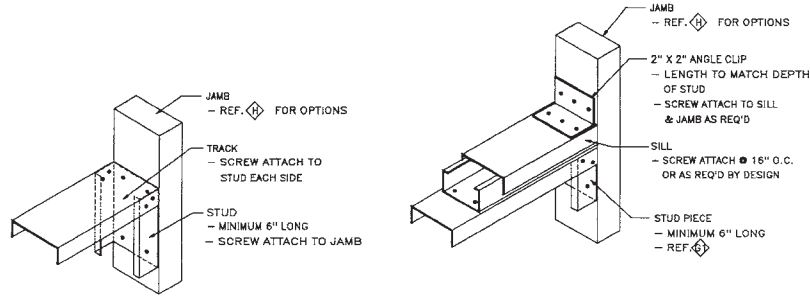
Forward

The framing connections shown herein are representative of typical construction practices. The user is cautioned to thoroughly evaluate the detail for specific performance and installation requirements.

The details primarily show screw attachments between framing components. Welds, sized and spaced as required, may be substituted for screws.



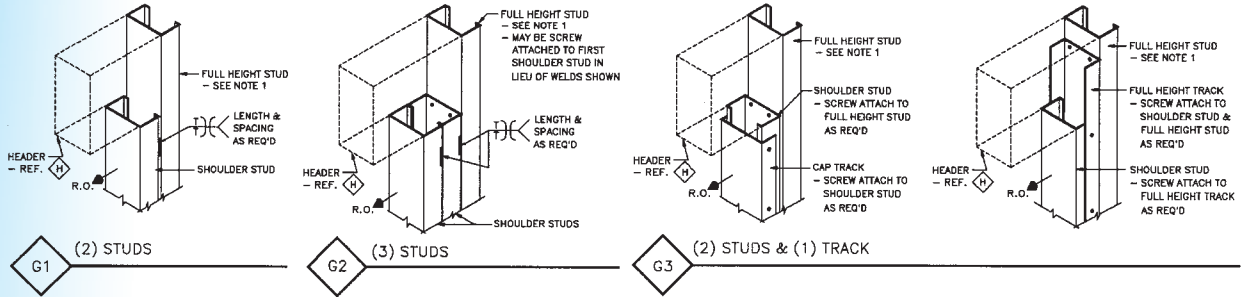
SUGGESTED CONNECTION DETAILS



G1 SILL CONSTRUCTION (1) TRACK

G2 SILL CONSTRUCTION (1) STUD & (2) TRACKS

G SILL ASSEMBLIES



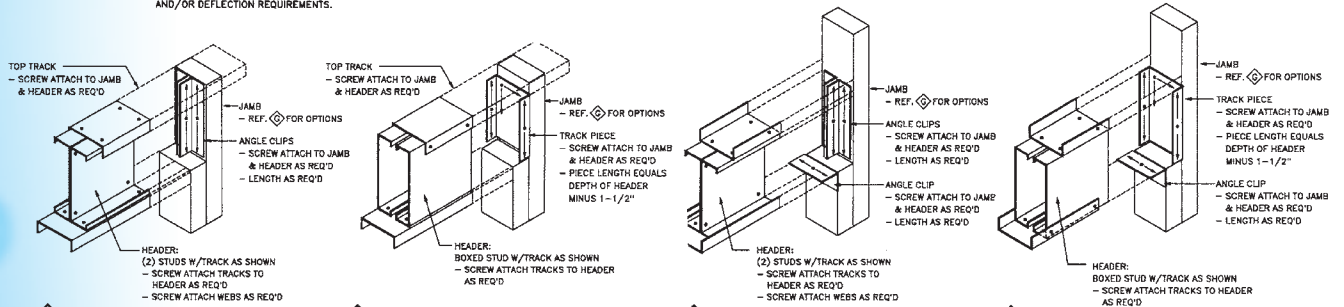
G1 (2) STUDS

G2 (3) STUDS

G3 (2) STUDS & (1) TRACK

G JAMB ASSEMBLIES

NOTE 1. EXTEND FULL HEIGHT STUD TO TOP OF WALL IF HEADER IS LOCATED DIRECTLY ABOVE OPENING (I.E. DROPPED HEADERS). ADDITIONAL FULL HEIGHT STUDS MAY BE REQUIRED TO MEET STRESS AND/OR DEFLECTION REQUIREMENTS.



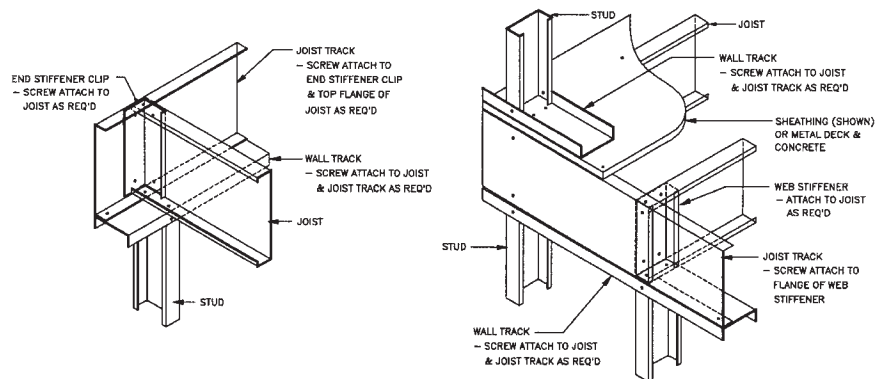
H1 "I" SHAPED HEADER @ TOP OF WALL

H2 BOXED HEADER @ TOP OF WALL

H3 "I" SHAPED HEADER ABOVE OPENING

H4 BOXED HEADER ABOVE OPENING

H HEADER ASSEMBLIES



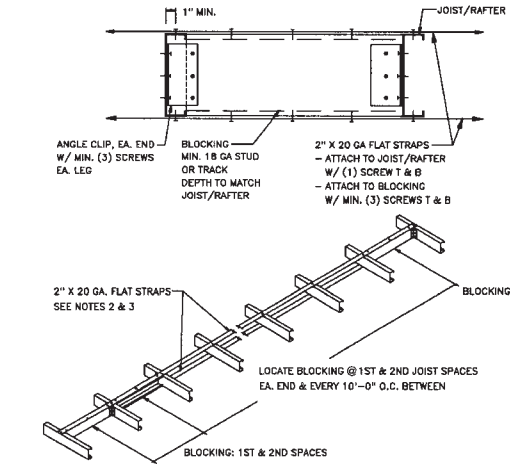
I1 WITH END STIFFENER CLIP

NOTE: ALIGN JOIST & STUD AS SHOWN. WHERE OFFSET IS ANTICIPATED, JOIST TRACK, SIZED AS REQUIRED, MAY BE USED TO TRANSFER JOIST REACTION

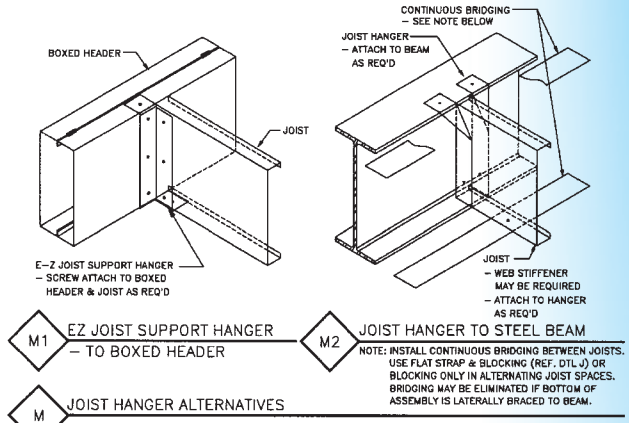
I2 WITH WEB STIFFENER

I JOIST END BEARING CONDITIONS

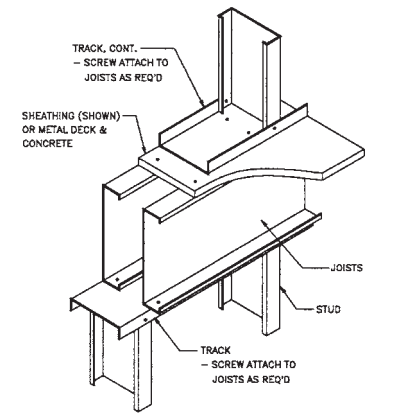
SUGGESTED CONNECTION DETAILS



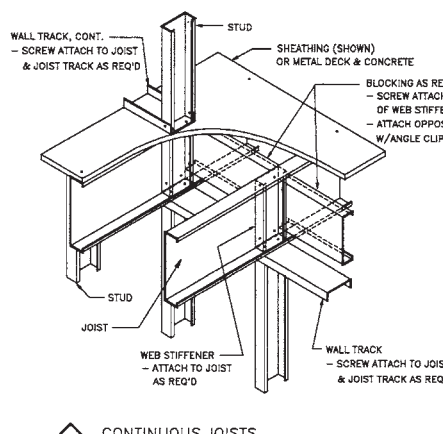
J JOIST OR RAFTER BRIDGING: FLAT STRAPS & BLOCKING
 NOTES: 1. LOCATE BRIDGING ROWS @ 7'-0" O.C. INTERVALS OR AS REQ'D BY DESIGN
 2. ATTACHMENT OF METAL DECK OR SHEATHING BEFORE LOADING ELIMINATES TOP STRAP
 3. REPLACE BOTTOM STRAP WITH 1-1/2" CHANNEL IF CEILING BOARD IS NOT DIRECTLY ATTACHED TO JOIST OR RAFTER



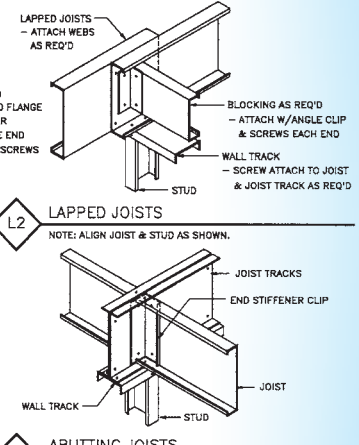
M1 EZ JOIST SUPPORT HANGER - TO BOXED HEADER
M2 JOIST HANGER TO STEEL BEAM
M JOIST HANGER ALTERNATIVES
 NOTE: INSTALL CONTINUOUS BRIDGING BETWEEN JOISTS. USE FLAT STRAP & BLOCKING (REF. D1LJ) OR BLOCKING ONLY IN ALTERNATING JOIST SPACES. BRIDGING MAY BE ELIMINATED IF BOTTOM OF ASSEMBLY IS Laterally BRACED TO BEAM.



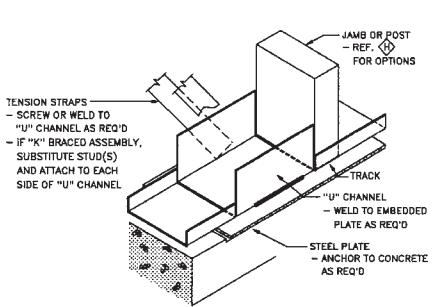
K JOISTS @ SIDE WALL



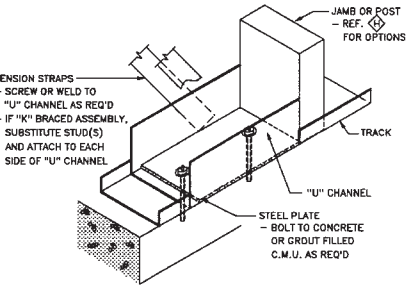
L1 CONTINUOUS JOISTS
 NOTE: ALIGN JOIST & STUD AS SHOWN.
L JOISTS OVER INTERIOR WALL



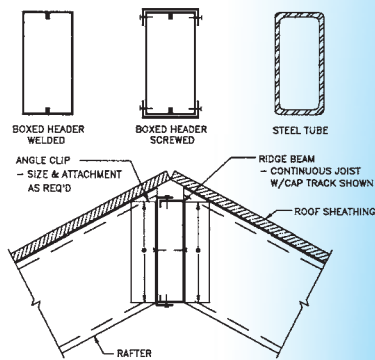
L2 LAPPED JOISTS
 NOTE: ALIGN JOIST & STUD AS SHOWN.
L3 ABUTTING JOISTS
 NOTE: ALIGN JOIST AND STUD AS SHOWN. WHERE OFFSET IS ANTICIPATED, JOIST TRACK, SIZED AS REQUIRED, MAY BE USED TO TRANSFER JOIST REACTION.



N1 WITH EMBEDDED STEEL PLATE
N SHEAR WALL BASE CONNECTIONS



N2 WITH BOLTED STEEL PLATE



O RAFTERS TO RIDGE BEAM
 (4) BEAM ALTERNATIVES SHOWN

GUIDE SPECIFICATION FOR A LOAD BEARING FRAMING SYSTEM

This information serves as a general guide to specifying a steel framed load bearing system. Italicized comments, where shown, serve as a commentary to the item it follows.

DIVISION 5

SECTION 05400-COLD FORMED STEEL FRAMING

1.0 GENERAL

1.01 Description of Work

Furnish all labor, materials and equipment as required to complete the installation of the cold formed steel framing system as described in the contract drawings and this specification.

1.02 Related Work

(List applicable specification sections for work which requires coordination with the steel framing system.)

1.03 Quality Assurance

1. Quality Control

a. Framing Identification

Each structural "C" stud component shall be identified with a factory applied marking denoting the manufacturer's name, mils and grade of steel. Studs manufactured from 33 KSI material shall be color coded with blue markings. Studs manufactured from 50 KSI steel shall be color coded red. The framing contractor is responsible for notifying the manufacturer in writing of this requirement.

b. Inspections

(Furnish requirements for shop and field inspections. Items to be considered include:

- Qualifications of inspection agency;
- Who bears the cost of the inspections;
- Intervals between inspections, etc.)

c. Qualifications

(Furnish qualification requirements of the framing contractor. Items to consider include:

- Provide documentation of past experience in the construction of steel framing systems similar in scope to the application shown on the contract documents;
- Qualification of welding procedures in accordance with requirements of the current edition of the American

Welding Society (AWS) "Specification for Welding Sheet Steel in Structures", D1.3).

d. Pre-Installation Conference

2. Standards

The design and installation of the steel framing system shall adhere to the minimum requirements of the current edition of the following specifications:

- a. American Iron and Steel Institute (AISI) "North American Specification for the Design of Cold Formed Steel Structural Members";
- b. American Welding Society (AWS) "Specification for Welding Sheet Steel in Structures", D1.3;
- c. American Society of Testing Materials (ASTM) Specifications:
 - C-955 "Standard Specification for Load Bearing (Transverse and Axial) Steel Studs, Runner (Track) and Bracing and Bridging, for Screw Application of Gypsum Wallboard and Metal Plaster Bases";
 - C-1007 "Standard Specification for the Installation of Load Bearing (Transverse and Axial) Steel Studs and Related Accessories".
- d. American Iron and Steel Institute (AISI) S200 and American Society of Testing Materials (ASTM) A1003.

1.04 Submittals

Prior to commencement of work, the contractor shall submit the following items for the approval of the Owner's Architect and Engineer of Record:

1. Drawings

For field assembled (stick-built) installations, the contractor shall provide drawings addressing the construction of each unique framing condition and connection. The drawings shall include descriptions, locations and spacings of each framing component and fastener.

For prefabricated applications, also include frame drawings depicting shape, dimensions, components, locations and construction sequence.

2. Structural Calculations

Submit calculations for review which shall include development of loading requirements and structural analysis of each unique framing and connection condition.

(The calculation requirement may be omitted if the Owner's Engineer performs the framing analysis. If calculations are

GUIDE SPECIFICATION

required, it is beneficial if the A/E establishes minimum design requirements including:

- Governing design code(s);
- Basic wind speed, exposure and importance classifications and seismic requirements;
- Floor and roof minimum live load requirements;
- Roof snow load requirement with provisions for drifting and sliding snow as dictated by code;
- Deflection limitations;
- Requirements for professional certification.)

3. Literature

Submit current technical literature prepared by the framing manufacturer. The structural properties and load tables shall be prepared in accordance with the contents of the current AISI's "North American Specification for the Design of Cold Formed Steel Structural Members".

4. Certifications

Submit statements prepared by the framing manufacturer certifying conformance with the minimum requirements of Section 2.0.

2.0 PRODUCTS

2.1 Acceptable Manufacturer

The cold formed steel framing products shall be furnished by SUPER STUD BUILDING PRODUCTS, INC., Edison, New Jersey or approved equal.

2.2 Materials

The steel framing products shall be manufactured from steel meeting the minimum requirements of the following specifications:

1. Mechanical Properties, Base Steel

Structural framing components shall be formed from steel meeting the minimum requirements of the following specifications:

	ASTM A1003 (F _y (min) = 50 KSI)	ASTM A1003 (F _y (min) = 33 KSI)
Studs	97, 68 & 54* mils	43 & 33 mils
Track & Accessories	97* & 68* mils	54*, 43 & 33 mils

F_y = Minimum Yield Point

* F_y=33KSI, 50 KSI must be specified at time of order.

2. Galvanized Coating

The steel framing products shall be zinc coated (galvanized) in accordance with ASTM A525, G-60.

(While a G-60 coating is our standard offering, Super Stud can also fulfill requests G-90 coatings which provides 50 percent more zinc protection.)

3.0 EXECUTION

3.1 General

1. Prefabricated wall and floor panels shall be square, with components attached in a manner as to prevent racking and to minimize distortion while lifting.
2. Cutting of steel framing members shall be by saw, shear or plasma-cutting equipment. Conventional oxyacetylene torch cutting is not permitted.
3. Storage and Handling
The steel framing components shall be stored on a flat plane, off ground and protected by impervious covering or shelter.
4. Sheathing boards shall be installed in accordance with most stringent requirement of the written specifications of the sheathing manufacturer or the current ASTM Specification addressing the same.
5. Temporary bracing shall be provided and remain in place until work is completely stabilized.
6. All framing components shall be plumbed, aligned and leveled.
7. Back blocking for wall mounted fixtures shall be designed and anchored to resist the applied loads.
8. Insulation equal to that specified elsewhere shall be furnished at multiple boxed members inaccessible to the insulation contractor.

3.2 Installation-Walls

1. Stud ends shall be square cut. Axially loaded single and multiple studs and header members shall be seated in the tracks to assure transfer of load. The maximum gap between the track web and end of the studs shall not exceed 1/8 inch.
2. Uniform and level bearing support shall be provided for the wall tracks. If not provided, install full size shims below bottom track at stud locations as needed or set bottom track in high-strength grout.
3. Studs shall be positioned to assure vertical alignment from level to level for the height of the building. Wall track shall not be used to transfer loads.
4. Vertical alignment shall be maintained at joist/truss and stud intersections or a load distribution member shall be provided to accommodate the offset loading condition.

In multi-level construction at conditions where the joists are not positioned to align with the bearing studs, a stub column consisting of members of equal size to the stud wall above shall be installed in the joist cavity.

5. Installation of shear wall assemblies must be completed before the attachment of sheathing products and floor and/or roof components. Diagonal flat straps shall be furnished de-coiled from the manufacturer.

GUIDE SPECIFICATION

6. Mechanical bridging shall be installed prior to the attachment of sheathing materials and loading. Bridging rows shall be spaced in accordance with the manufacturer's recommendations or as dictated by structural analysis.

7. Splicing of framing, other than the continuous track at the top and bottom of the wall, is not permitted.

Where track ends abut between stud, joist and/or rafter spacings, a piece of stud shall be placed in the adjoining tracks and fastened to the flanges, both sides, each end. Where track ends abut at studs, joists or rafters, attach each end to the member, both sides.

8. Multiple framing components required at posts or jambs, sills and head conditions of framed openings, shall be assembled with intermittent connections between the members.

3.3 Installation-Roof and Floor Joists

1. Do not overload the roof or floor system during construction. Place heavy loads, materials, equipment, etc. directly over structural supports, bearing walls or as dictated by structural analysis.

2. End blocking shall be provided where joist ends are not otherwise restrained from rotation.

3. Provide additional floor joists under parallel non-load bearing partitions where the partition length exceeds 1/2 the joist span unless otherwise verified by structural analysis.

4. Web stiffeners shall be provided at support locations and at points of concentrated loads unless specifically shown otherwise herein.

5. Web knockouts shall not be located within distance equalling 1.5 times the depth of the member (h) from edge of bearing. Where this provision is unavoidable, web reinforcement shall be provided.

6. Joist bridging shall be provided where indicated on the contract documents or approved shop and erection drawings.

7. Maintain a minimum 1-3/4" bearing at joist ends.

8. Where joists are flush mounted to support beams, furnish clip angles or hangers rated for the joist reaction with an applicable factor of safety. Where hangers are provided, install blocking as required to prevent joist rotation.

3.4 Connections

1. General

a. Fasteners shall be designed and installed in accordance with the manufacturer's written instructions or industry accepted standards.

b. Concrete fasteners shall be installed after the design compressive strength of the concrete is obtained.

2. Fasteners Types

a. Framing Screws

Screw penetration through joined materials shall not be less than three exposed threads. Where screw attachments are made to framing components of different thicknesses, attachment through the thinner component to the heavier is required. Select screws with an adequate cutting tip to accommodate the total thickness to be drilled. Drilling must be completed before the threads engage the material.

b. Welds

Welded connections shall be performed in accordance with the current edition of the AWS D1.3 Specification for Welding Sheet Steel in Structures. Consult AWS Publication WZC, Welding Zinc Coated Steels, discussing arc welding procedures and safe practices.

Minimum weld throat thicknesses must match or exceed the base metal thickness of the thinnest connected part unless noted otherwise.

Welds shall be cleaned and painted with rust inhibiting galvanizing paint.

(Field welds involving 20 gage materials should be avoided.)

c. Powder Actuated Fasteners (PAF)

To Steel: PAF's used for the attachment of the framing system to hot rolled steel components shall possess knurled shanks. Full tip penetration through the steel component is required. A 3/4 inch minimum edge distance shall be maintained.

To Concrete: PAF's used for the attachment of the framing system to concrete shall be of adequate length to assure minimum embedment requirements. Unless noted otherwise, a 3 inch minimum edge distance shall be maintained. Multiple fasteners shall not be spaced less than 4 inches apart.

d. Expansion Bolts or Adhesive Anchors to Concrete

Anchors shall be installed in accordance with the manufacturer's current printed instructions.

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